



STATE	PROJECT	SHEET	TOTAL	
OF		NO.	SHEETS	
S.D.				

## 3

#### **ESTIMATE OF STRUCTURE QUANTITIES**

DESCRIPTION	QUANTITY	UNIT	REMARKS
Concrete Penetrating Sealer	1,016	SqYd	See Special Provision
Incidental Work, Structure	Lump Sum	LS	
Membrane Sealant Expansion Joint	146.7	Ft	
Structure Excavation, Bridge	1308	CuYd	
Bridge End Embankment	357	CuYd	
Granular Bridge End Backfill	113.8	CuYd	
Approach Slab Underdrain Excavation	5.3	CuYd	
Class A45 Concrete, Bridge Deck	599.3	CuYd	
Class A45 Concrete, Bridge	434.3	CuYd	
Concrete Approach Slab for Bridge	277.7	SqYd	
Concrete Approach Sleeper Slab for Bridge	97.3	SqYd	
Deck Drain, Slab Bridge	8	Each	
Controlled Density Fill	12.9	CuYd	
Steel Pedestrian Railing on Sidewalk	391.0	Ft	
Steel Pedestrian Railing on Concrete Barrier	308.0	Ft	
Reinforcing Steel	82,277	Lb	
Epoxy Coated Reinforcing Steel	155,278	Lb	
Extract Pile	81	Each	
Preboring Pile	160	Ft	
HP 10x42 Steel Test Pile, Furnish and Drive	490	Ft	
HP 10x42 Steel Bearing Pile, Furnish and Drive	10,140	Ft	
4" Rigid Galvanized Steel Conduit	310	Ft	
6" Reinforced Concrete Sidewalk	544	SqFt	
4" Underdrain Pipe	437	Ft	
Porous Backfill	47.8	Ton	
Class B Riprap	970.1	Ton	
Type B Drainage Fabric	928	SqYd	

## 4

#### **SPECIFICATIONS FOR BRIDGE**

- Design Specifications: AASHTO LRFD Bridge Design Specifications, 2017 Edition.
- 2. Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, 2015 Edition and required provisions, supplemental specifications, and special provisions as included in the proposal.

#### **BRIDGE DESIGN LOADING**

- 1. AASHTO HL-93.
- 2. Dead Load includes 22 psf for future wearing surface on the roadway.

#### **DESIGN MATERIAL STRENGTHS**

Concrete f'c = 4,500 psi Reinforcing Steel fy = 60,000 psi Piling (ASTM A572 Grade 50) fy = 50,000 psi

#### **GENERAL CONSTRUCTION**

- 1. All mild reinforcing steel shall conform to ASTM A615, Grade 60.
- All exposed concrete corners and edges shall be chamfered 3/4" unless noted otherwise.
- 3. Use 2" clear cover on all reinforcing steel except as shown.
- 4. Contractor shall imprint on the structure the date of new construction as specified and detailed on Standard Plate No. 460.02.
- 5. Barrier Curbs shall be built normal to the grade.
- 6. Request for construction joints or re-steel splices at points other than those shown, must be submitted to the Engineer for prior approval. If additional splices are approved, no payment will be allowed for the added quantity of re-steel.
- 7. The elevation of the bridge deck is 21.5" above subgrade elevation.

#### **INCIDENTAL WORK, STRUCTURE**

- 1. In place centerline Sta. 23+50.80 to centerline Sta. 24+96.80 is a 146.0' 4 span continuous concrete slab bridge with a 54'-0" clear roadway. The superstructure consists of a reinforced concrete slab with concrete barrier curb continuous across the bridge and 5'-8" cantilever sidewalk with steel railing on both sides of the bridge. The deck includes a 2 inch low slump dense concrete overlay and a single layer epoxy chip seal overlay. The substructure consists of reinforced concrete pier walls and reinforced concrete vertical abutments, all of which are supported on timber piling. The east abutment includes sheet pile in front of the berm and a portion of the wingwall of the adjacent dam. A 26 ½" sanitary sewer pipe is located approximately 27 feet north of the roadway centerline. The pipe is to be relocated by the City of Watertown.
- 2. Break down and remove the existing bridge, and approach/sleeper slabs if applicable, to 1 foot below finished groundline, or as required to construct the new structure in accordance with Section 110 of the Specifications. All portions of the existing bridge shall be removed and disposed of by the Contractor on a site obtained by the Contractor and approved by the Engineer in accordance with the Environmental Commitments found in Section A. A portion of the wingwall of the adjacent dam shall be removed to construct Abutment No. 5. See Abutment No. 5 Details (A).
- 3. During demolition of the structure, efforts shall be taken to prevent material from falling into the river. Under no circumstances is asphalt allowed to fall into the river.
- 4. The foregoing is a general description of the in-place bridge and should not be construed to be complete in all details. Before preparing the bid it shall be the responsibility of the Contractor to make a visual inspection of the structure to verify the extent of the work and materials involved. If desired by the Contractor, a copy of the original construction plans may be obtained through the Office of Bridge Design.

5. It is anticipated that at least 81 timber piles will interfere with piling for this new structure. Any existing pile that interferes with piling for the new structure shall be extracted. Payment for the extracting piling shall be contract unit price per each for Extract Pile and shall be full compensation for extracting piling including materials, labor, and equipment necessary or incidental to the satisfactory completion of this work.

#### **DESIGN MIX OF CONCRETE**

- 1. All structural concrete shall be Class A45 unless otherwise indicated.
- 2. Type II cement is required.

#### **ABUTMENTS**

- 1. Pre-boring piling at each abutment is required to whichever is greater, ten feet or to natural ground
- 2. The HP 10x42 Piling were designed using a factored bearing resistance of 77 tons per pile. Piling shall develop a field verified nominal bearing resistance of 192 tons per pile.
- 3. The contractor shall have sufficient pile splice material on hand before pile driving is started. See Standard Plate No. 510.40.
- 4. Piles shall not be driven out of position by more than three inches in the direction normal to the abutment centerline. A pile-driving template shall be used to ensure this accuracy.
- 5. One test pile shall be driven at each abutment and will become part of the pile group.
- 6. Each finished abutment shall include a Bridge Survey Marker. See Standard Plate No. 460.05.

#### **REQUIRED LIST**

(1) Title Block

2 Project Block

3 Estimate of Quantities

4 Notes

1 ESTIMATE OF STRUCTURE QUANTITIES AND NOTES
FOR

155' - 0" CONT. CONCRETE BRIDGE

STR. NO. 15-181-180 SEPTEMBER 2017



DESIGNED BY CK. DES. BY DRAFTED BY SERIES BRIDGE ENGINEER

# STATE PROJECT SHEET TOTA NO. SHEE

### 3 PILE DRIVING

1. A drivability analysis was performed using the wave equation analysis program (GRLWEAP). The following pile hammers were evaluated and found to produce acceptable driving stresses:

Delmag D25-32 Delmag D30-32

SPI D30 APE D30-52

2. Pile hammers not listed will require evaluation and approval prior to use from the Geotechnical Engineering Activity.

#### **PIERS**

- The HP 10x42 Piling were designed using a factored bearing resistance of 77 tons per pile. Piling shall develop a field verified nominal bearing resistance of 192 tons per pile.
- One test pile shall be driven at each bent and will become part of the pile group.
- 3. The contractor shall have sufficient pile splice material on hand before pile driving is started. See Plate No. 510.40
- 4. It is anticipated that cofferdams and foundation seals will be necessary. Cofferdams and foundation seals shall be designed and constructed in accordance with Section 423 of the Specifications. Contact the Office of Bridge Design if additional piles are required.
- 5. Galvanize the pier wall armoring plates and anything welded to them after all welding is completed. They shall be galvanized in accordance with AASHTO M111 (ASTM A123). If welded splices are used subsequent to galvanizing, the weld details and the procedures for preparing the surface for welding and repairing the galvanizing after welding shall be included with the shop plans. Repair of galvanizing shall be by zinc-based solder method in conformance with ASTM A780.
- 6. Steel for the pier wall armoring plates shall be ½" thickness and conform to ASTM A709, Grade 36. Shear connectors shall conform to Clause 7.3, Type A or b of the AASHTO/AWS D1.5 Bridge Welding Code.
- 7. Welding for the pier wall armoring plates shall be in accordance with AWS D1.1 Structural Welding Code Steel.
- 8. The cost of the pier wall armoring plates complete and in-place including fabrication, welding, and galvanizing shall be incidental to the contract unit price per cubic yard for Class A45 Concrete, Bridge.

#### SUPERSTRUCTURE

 Preplanned construction joints may be used in accordance with Section 460.3 of the Specifications. Contact the Office of Bridge Design for joint configuration and allowable location. Emergency slab construction joints shall be as shown with the superstructure details. If an emergency slab joint is used, contact the Office of Bridge Design before proceeding with deck pour.

- The deck-finishing machine shall be adjusted and operated in such a
  manner that the roller screed or screeds are parallel with the
  centerline of the bridge and the finish machine is parallel to the skew
  of the bridge. Concrete placement in front of the finish machine shall
  be kept parallel to the machine.
- 3. Barrier curbs shall be poured after all the slab has been poured. Superstructure falsework shall not be removed until bridge deck concrete, including barrier curbs, has attained a strength of 2400 psi.
- 4. The bridge deck must be placed and finished continuously at a minimum rate of 22 ft. of deck per hour measured along centerline roadway. If concrete cannot be placed and finished at this rate, the Engineer shall order a header installed and operations stopped. Notify the Bridge Construction Engineer if deck pour operations are stopped. Operations may resume only when the Engineer is satisfied that a minimum rate of 22 ft. of deck per hour can be achieved and the concrete in the previous pour has attained a minimum compressive strength of 2000 psi.
- 5. Snap ties, if used in barrier curb formwork, shall be epoxy coated. The epoxy coating shall be inert in concrete and compatible with the coating applied to the new epoxy coated reinforcing steel.

#### **CLASS A45 CONCRETE, BRIDGE DECK**

- 1. Concrete used in the bridge deck slab and barrier curbs shall be in accordance with the requirements for bridge deck concrete as specified in Section 460.3 A of the Specifications.
- 2. See Special Provision for Concrete Penetrating Sealer.

#### **CLASS B COMMERCIAL TEXTURE FINISH**

- A Class B commercial texture finish shall be applied to the following areas:
  - \*Abutments: all exposed surfaces to an elevation 1-foot below finished ground line.
  - b) Barrier Rail: all exposed surfaces (\*\*front, \*\*top and \*back).
  - c) \*Slab: edge of slab.
  - d) \*Piers: all exposed surfaces.

\*Color shall be an approved tan.

- \*\*Color shall be "Pearl White."
- 2. The Class B commercial texture finish shall be applied in accordance with Section 460.3 L.1.c of the Specifications.
- 3. Where the Class B commercial texture finish is to be applied, concrete curing shall be accomplished with cotton or burlap mats and polyethylene sheeting. Curing shall continue for not less than seven days after placing concrete before the commercial texture finish is applied. The commercial texture finish shall be applied in accordance with the manufacturer's recommendations. The commercial texture finish itself does not require a specific cure except for drying.

#### **APPROACH SLABS**

- 1. Sleeper slab riser shall be cast with the approach slab or cast after the approach slab is placed. Care shall be taken to ensure the correct grade is maintained across the joint.
- 2. The portion of the sleeper slab below the construction joint may be precast. If the bottom portion of the sleeper slab is precast, the Contractor shall submit proposed lifting and setting plans to the Bridge Construction Engineer for approval. In addition, if reinforcing or other details differ from those shown in the plans, the Contractor shall submit proposed alternate details for approval.
- 3. The use of an approved finishing machine will be required during placement of Class A45 Concrete for the approach slabs. Concrete placement in front of the machine shall be kept parallel to the screed.
- 4. The concrete in the approach slab shall be tined normal to centerline roadway.
- 5. Concrete Approach Sleeper Slab for Bridge, whether cast-in-place or precast, will be paid for at the contract unit price per square yard. This payment shall be full compensation for all excavation, furnishing, hauling, and placing all materials including concrete and reinforcing steel; for disposal of all excavated material and surplus materials; and for labor, tools, equipment and any incidentals necessary to complete this item of work.
- 6. Concrete Approach Slab for Bridge will be paid for at the contract unit price per square yard. This payment shall be full compensation for all excavation, furnishing, hauling and placing all materials including concrete, asphalt paint or 6 mil polyethylene sheeting, elastic joint sealer and reinforcing steel; for disposal of all excavated material and surplus materials and for labor, tools, equipment and any incidentals necessary to complete this item of work.

# REQUIRED LIST 1 Title Block 2 Project Block 3 Notes



155' - 0" CONT. CONCRETE BRIDGE

STR. NO. 15-181-180 SEPTEMBER 2017



DESIGNED BY CK. DES. BY DRAFTED BY STEVE A JAMES OF BRIDGE ENGINEER

# STATE PROJECT SHEET TOTAL NO. SHEETS

## 3

#### **DECK DRAINS**

- Deck Drains shall be 4" diameter by 1' 6 ½" Schedule 40 Polyvinyl Chloride (PVC) Plastic Pipe conforming to the requirements of ASTM D1785
- 2. A 4 1/2 inch diameter by 2 inch PVC Plastic Pipe Sleeve conforming to the requirements of ASTM D1785 shall be attached to the 4" diameter PVC Pipe, as shown in the plans, with a solvent cement conforming to ASTM D2564.
- Payment for Deck Drains shall be at the contract unit price per each for Deck Drain, Slab Bridge, and shall be full compensation for furnishing, fabricating and installing the deck drains in accordance with the Plans and Specifications.
- 4. The location of the deck drains may be adjusted slightly to clear transverse slab steel.

#### **STEEL RAILING - SIDEWALK**

- 1. All rail posts shall be built vertical.
- 2. All structural steel parts for railing shall conform to ASTM A500, Grade B. Material less than ¼" thick may be ASTM A1011, Grade 36. Rail post base plates shall conform to ASTM A709, Grade 36.
- 3. All anchor bolts and nuts for railing shall conform to ASTM A307. Washers shall conform to ASTM F436 and all components shall be galvanized in accordance with ASTM A153 or ASTM F2329, as applicable. The bolts shall be hex head "structural" type with heavy hex nuts and round washers.
- 4. All anchor bolts shall be tightened to a torque of 120 ft.-lbs. (approximated without the use of a calibrated torque wrench).
- 5. The non-shrink grout used to fill the recess beneath the rail post base plates shall be a commercially available non-shrink grout containing no metallic particles and capable of attaining a 28 day compressive strength of 3000 psi. The non-shrink grout shall be mixed according to the manufacturer's recommendations. The cost of furnishing and placing the non-shrink grout shall be incidental to the contract unit price per foot for Steel Pedestrian Railing on Sidewalk.
- 6. All steel railing shall be galvanized after shop welding in accordance with ASRM A123 and shall be painted in accordance with Section 411 of the Specifications and the color shall be an approved black (Federal Standard 595B Color 27038). The galvanized steel railing shall be cleaned in accordance with ASTM D6386 before painting.
- 7. Welding & Weld Inspection shall be done in accordance with the current edition of AWS D1.1 Structural Welding Code-Steel.
- 8. The costs of structural steel, welding, weld inspection, painting and galvanizing shall be incidental to the contract unit price per foot for Steel Pedestrian Railing on Sidewalk and Steel Pedestrian Railing on Concrete Barrier.

#### SIDEWALK APPROACH SLABS

- 1. The reinforced concrete sidewalks adjacent to the bridge shall be paid for at the contract unit price per square foot for 6" Reinforced Concrete Sidewalk. This payment will be full compensation for all excavation, furnishing, hauling and placing all materials including concrete, epoxy coated reinforcing steel, asphalt paint or 4 mil polyethylene sheeting, hot poured elastic joint sealer; for disposal of all excavated and surplus materials; and for all labor, tools, equipment and incidentals necessary to complete this item of work.
- 2. The top of the sidewalk shall transition from the end of the bridge to the top of approach slab curb at the sidewalk expansion device.
- 3. All costs involved in furnishing and placing the sidewalk sleeper slabs shall be included in the contract unit price per square foot for 6" Reinforced Concrete Sidewalk.

#### **NOTICE - LEAD BASED PAINT**

Be advised that the paint on the steel surfaces of the existing structure contains lead. The Contractor should plan his/her operations accordingly, and inform his/her employees of the hazards of lead exposure.

#### 4" RIGID GALVANIZED STEEL CONDUIT

- 1. The ¼" diameter concrete inserts for conduit clamps shall be commercially available inserts threaded for use with a galvanized ¼" diameter A307 bolt. The insets shall be capable of developing the strength of A307 bolt and shall be galvanized or stainless steel. The cost of furnishing and installing the inserts shall be incidental to the contract unit price per cubic yard for Class A45 Concrete, Bridge Deck.
- 2. The cost for furnishing and installing the 5" sleeves and polyethylene sheeting shall be incidental to the contract unit price per cubic yard for Class A45 Concrete, Bridge.

#### **REQUIRED LIST**

1 Title Block

2 Project Block

3 Notes

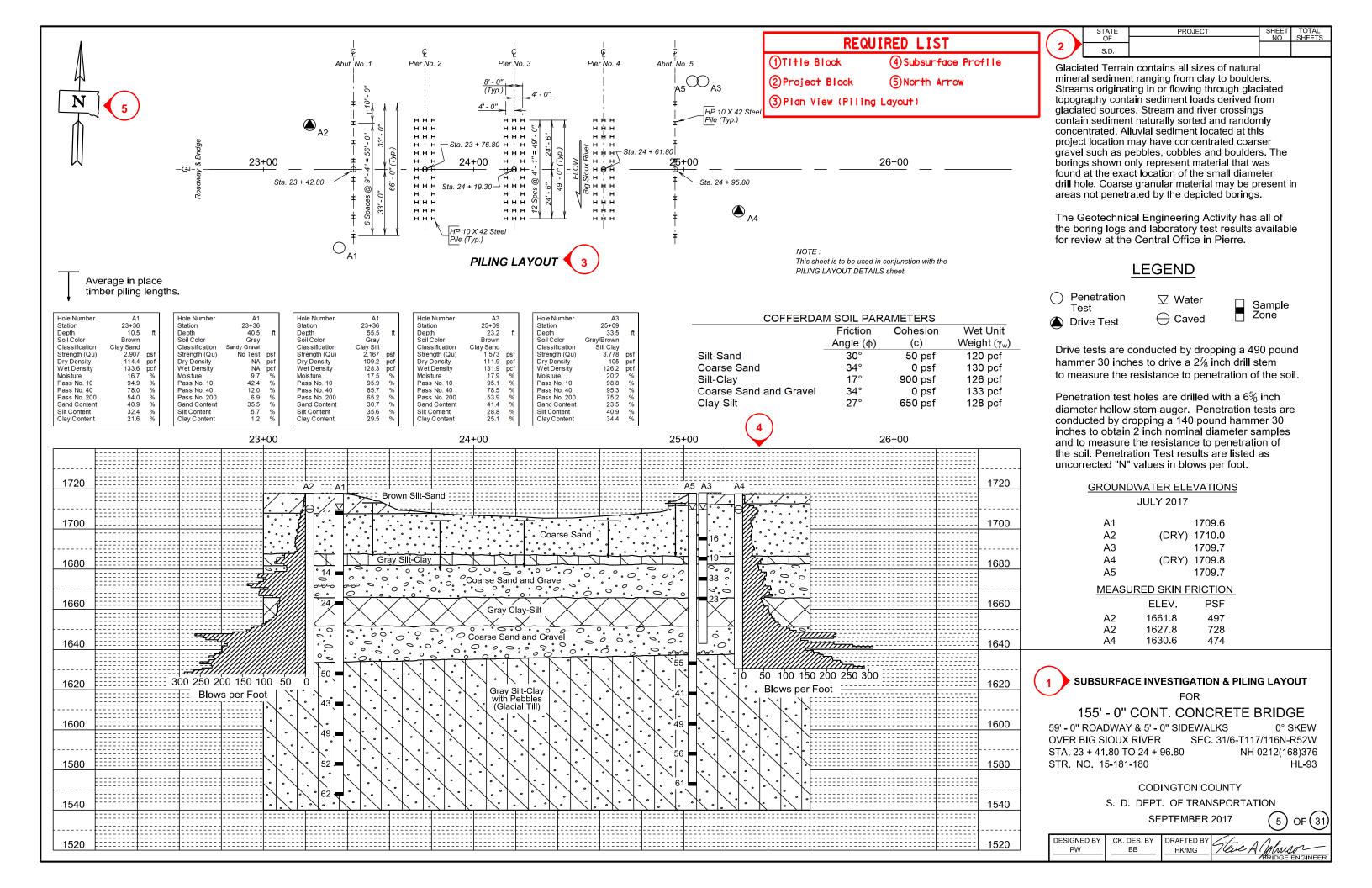


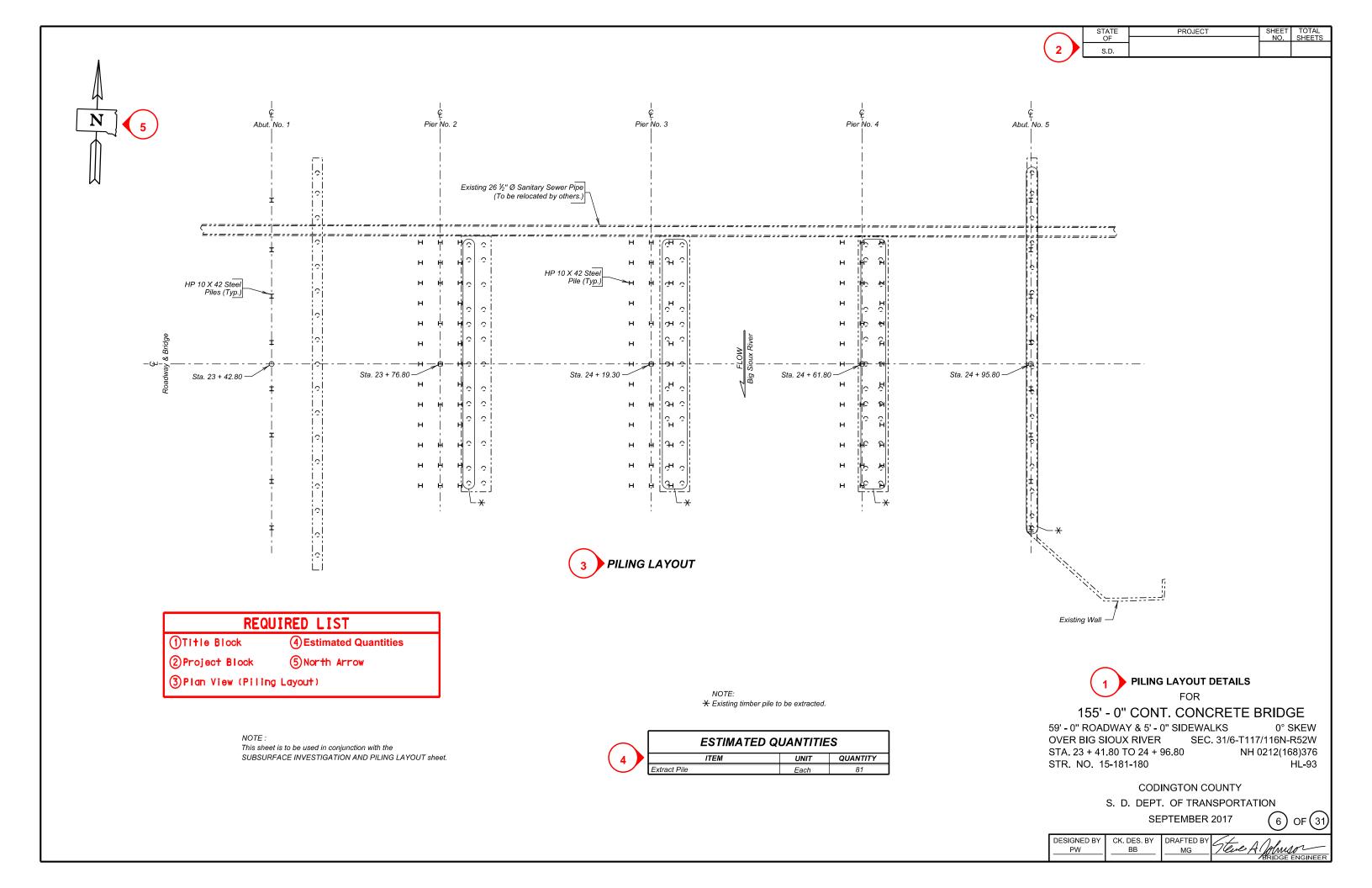
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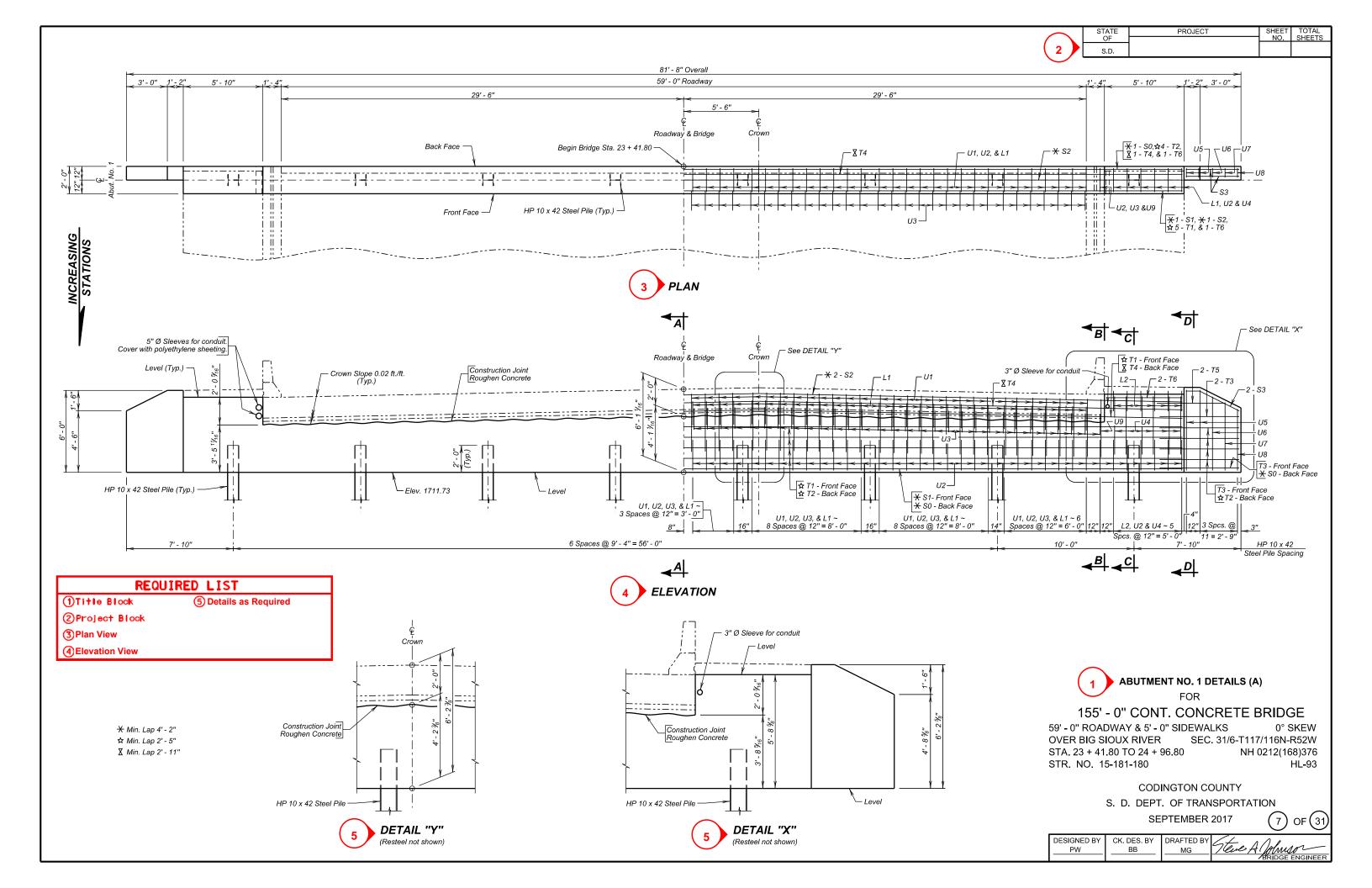
STR. NO. 15-181-180 SEPTEMBER 2017

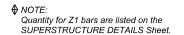


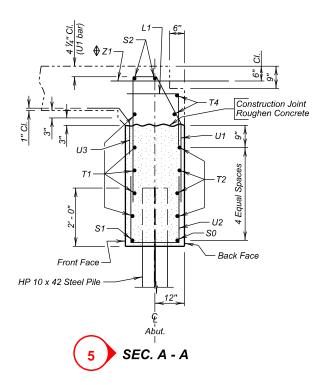
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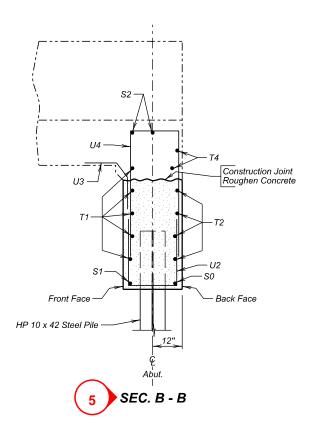


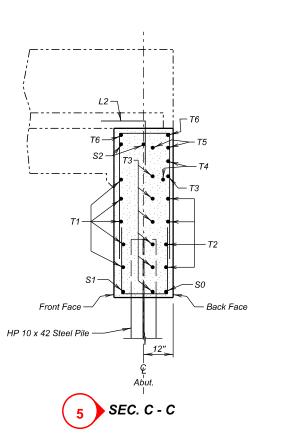


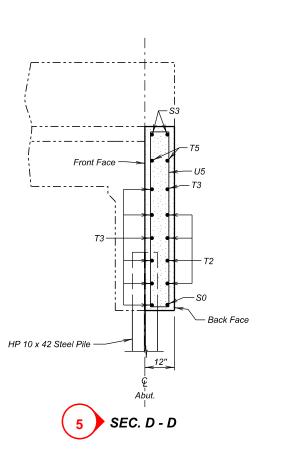




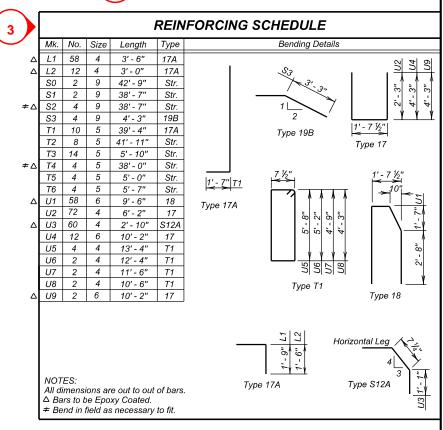








# STATE OF PROJECT SHEET TOTAL NO. SHEETS S.D.





ESTIMATED QUANTITIES			
ITEM	UNIT	QUANTITY	
Class A45 Concrete, Bridge	Cu. Yd.	24.3	
Reinforcing Steel	Lb.	2062	
Epoxy Coated Reinforcing Steel	Lb.	1815	
Structure Excavation, Bridge	Cu. Yd.	15.4	
HP 10 x 42 Steel Test Pile, Furnish & Drive	Ft.	1 @ 110' = 110'	
HP 10 x 42 Steel Bearing Pile, Furnish & Drive	Ft.	7 @ 105' = 735'	
Preboring Pile	Ft.	8 @ 10' = 80'	

#### REQUIRED LIST

1)Title Block

(5) Sections as Required

2 Project Block

3Reinforcing Schedule

4 Estimated Quantities



#### 155' - 0" CONT. CONCRETE BRIDGE

59' - 0" ROADWAY & 5' - 0" SIDEWALKS 0° SKEW
OVER BIG SIOUX RIVER SEC. 31/6-T117/116N-R52W
STA. 23 + 41.80 TO 24 + 96.80 NH 0212(168)376
STR. NO. 15-181-180 HL-93

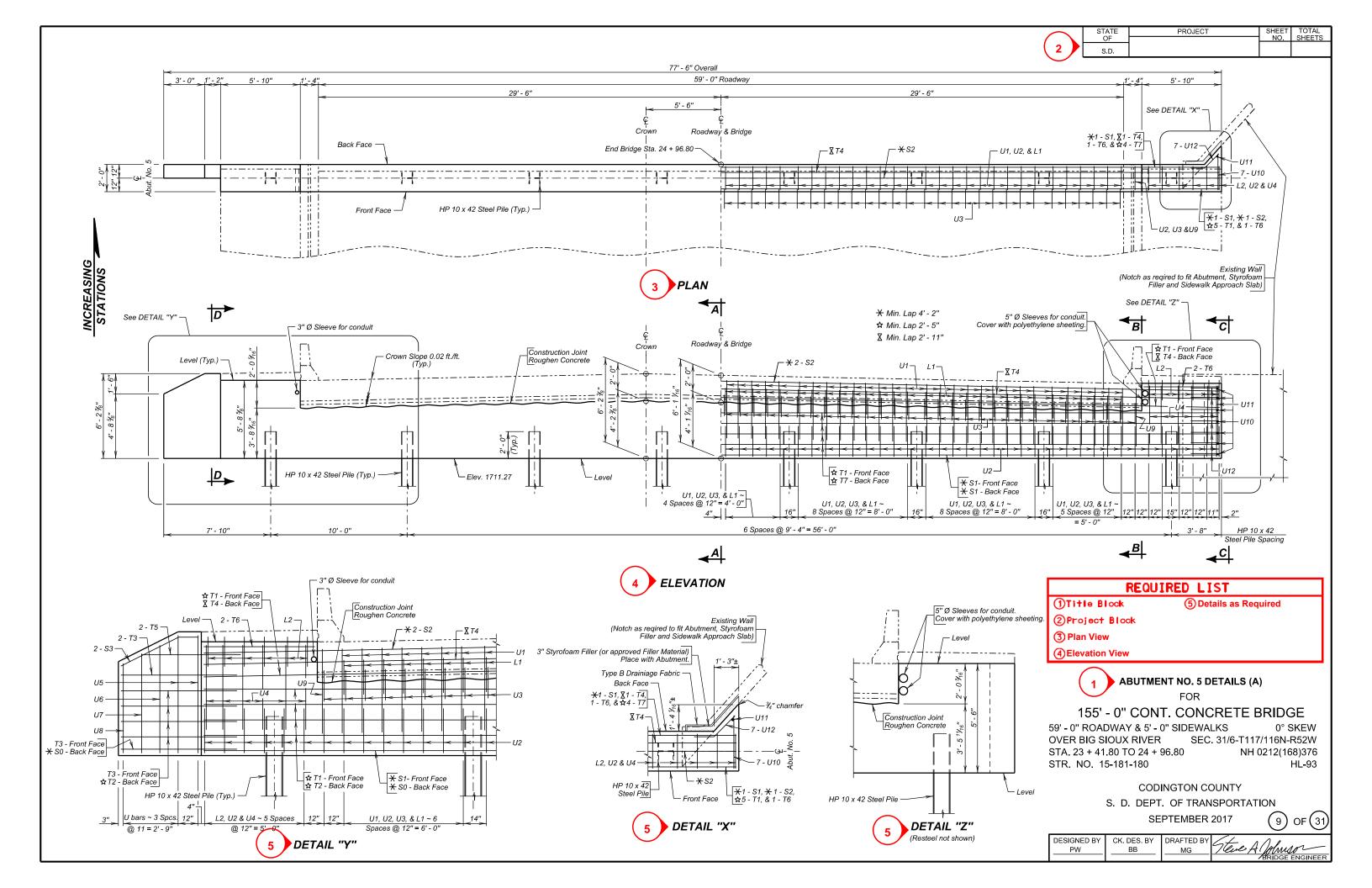
CODINGTON COUNTY

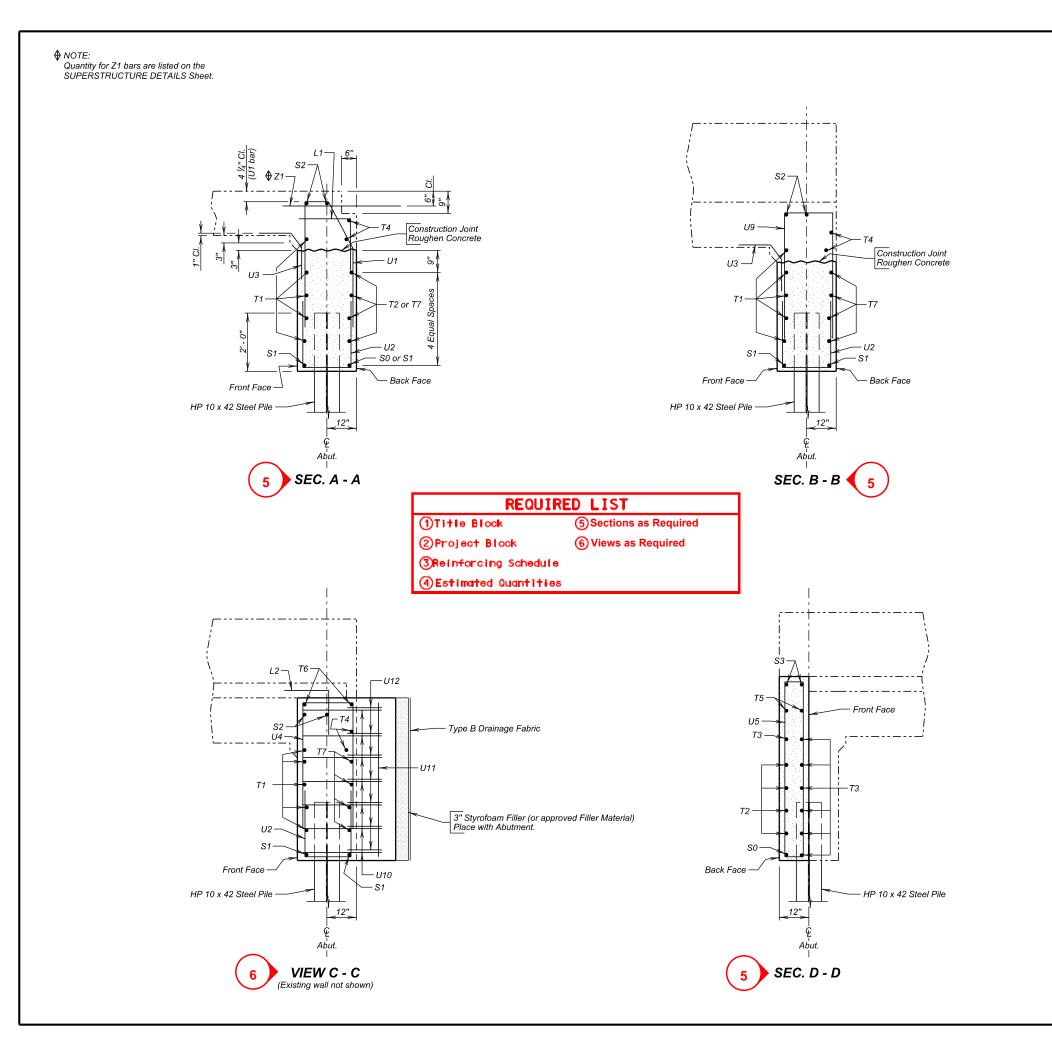
S. D. DEPT. OF TRANSPORTATION

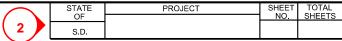
SEPTEMBER 2017



DESIGNED BY	CK. DES. BY	DRAFTED BY	Steve A Johnson
PW	BB	MG	
			BRIDGE ENGINEER







REINFORCING SCHEDULE 3 Bending Details Mk. No. Size Length Type L1 58 4 3'-6" 17A L2 12 4 3'-0" 17 S0 1 9 42'-9" Str. S1 3 9 38'-7" Str. S2 4 9 38'-7" Str. S3 2 9 4'-3" 19B T1 10 5 39'-4" 17A T2 4 5 41'-11" Str. 1'-71/2" T3 7 5 5'-10" Str. Type 19B T4 4 5 38'-0" Str. Type 17 T5 2 5 5' - 0" Str. 1' - 7" T1 T6 4 5 5' - 7" T7 4 5 38'-0" Str. Type 2 U1 | 58 | 6 | 9'-6" | 18 | U2 72 4 6' - 2" U3 60 4 2'-10" S12A U4 12 6 10'-2" 17 U5 2 4 13' - 4" U6 1 4 12'-4" T1 U7 | 1 | 4 | 11'-6" | T1 U8 1 4 10'-6" U9 2 6 10'-2" 17 Type 18 U10 7 5 2'-8" Str. U11 1 5 5' - 2" Str.
U12 7 5 2' - 5" 19B Type T1 Horizontal Leg Type S12A NOTES: Type 17A All dimensions are out to out of bars. △ Bars to be Epoxy Coated.



≠ Bend in field as necessary to fit.

ESTIMATED QUANTITIES			
ITEM	UNIT	QUANTITY	
Class A45 Concrete, Bridge	Cu. Yd.	23.6	
Reinforcing Steel	Lb.	1951	
Epoxy Coated Reinforcing Steel	Lb.	1815	
Structure Excavation, Bridge	Cu. Yd.	15.0	
HP 10 x 42 Steel Test Pile, Furnish & Drive	Ft.	1 @ 110' = 110'	
HP 10 x 42 Steel Bearing Pile, Furnish & Drive	Ft.	7 @ 105' = 735'	
Preboring Pile	Ft.	8 @ 10' = 80'	

☐ The cost for furnishing and installing Styrofoam filler and Type B Drainage Fabric shall be incidental to the contract unit price per cubic yard for Class A45 Concrete, Bridge.



#### 155' - 0" CONT. CONCRETE BRIDGE

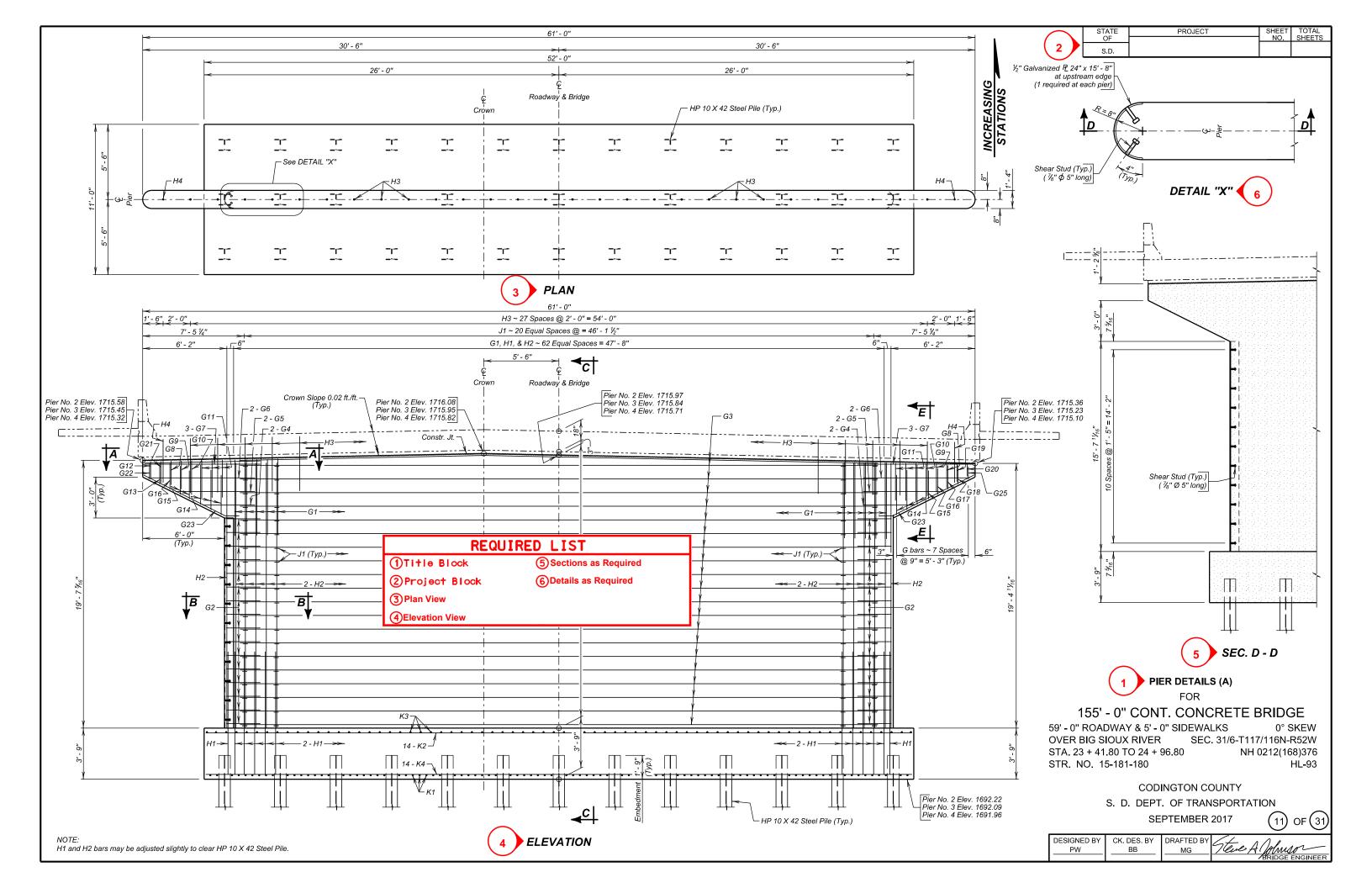
59' - 0" ROADWAY & 5' - 0" SIDEWALKS 0° SKEW
OVER BIG SIOUX RIVER SEC. 31/6-T117/116N-R52W
STA. 23 + 41.80 TO 24 + 96.80 NH 0212(168)376
STR. NO. 15-181-180 HL-93

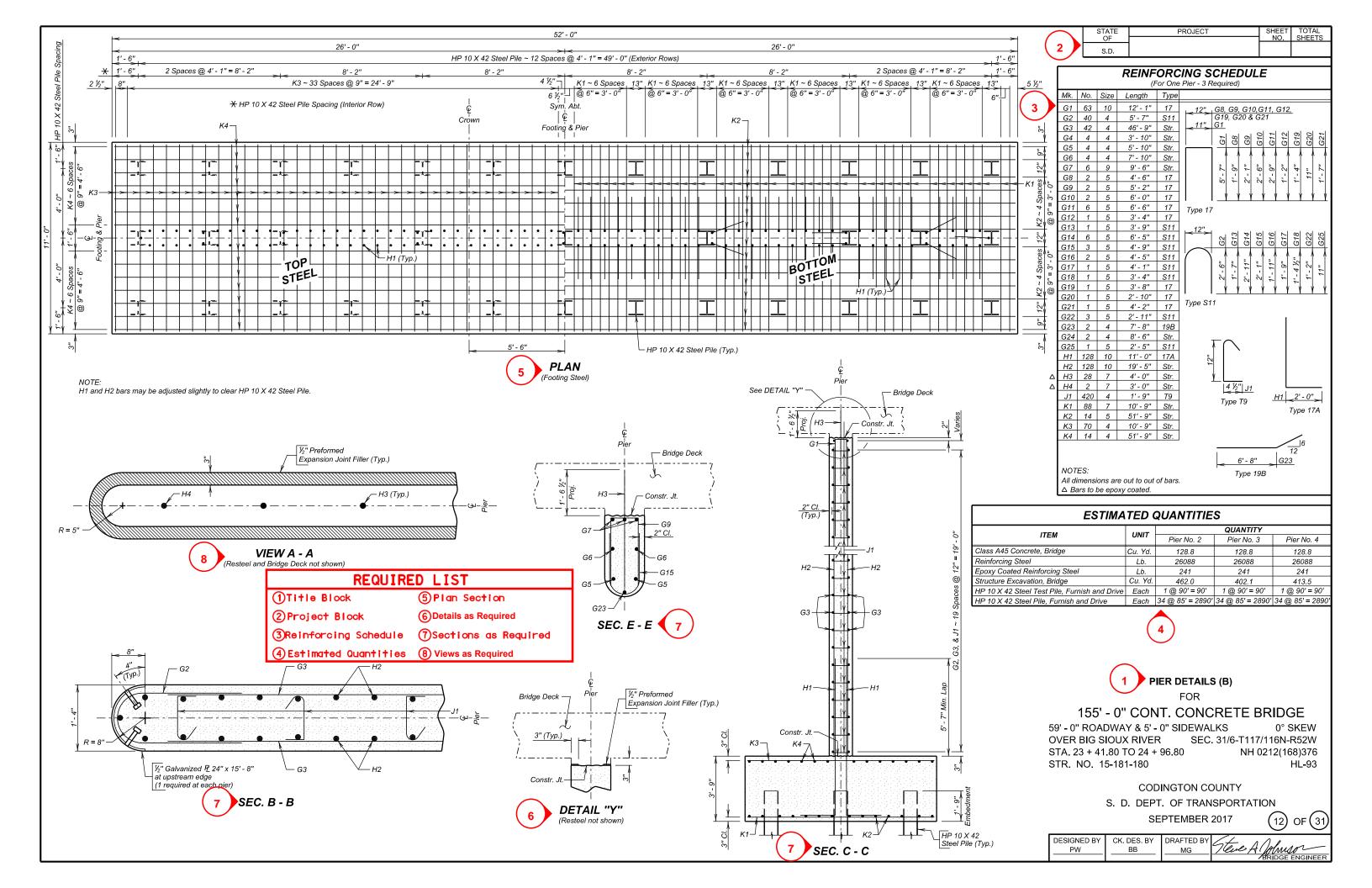
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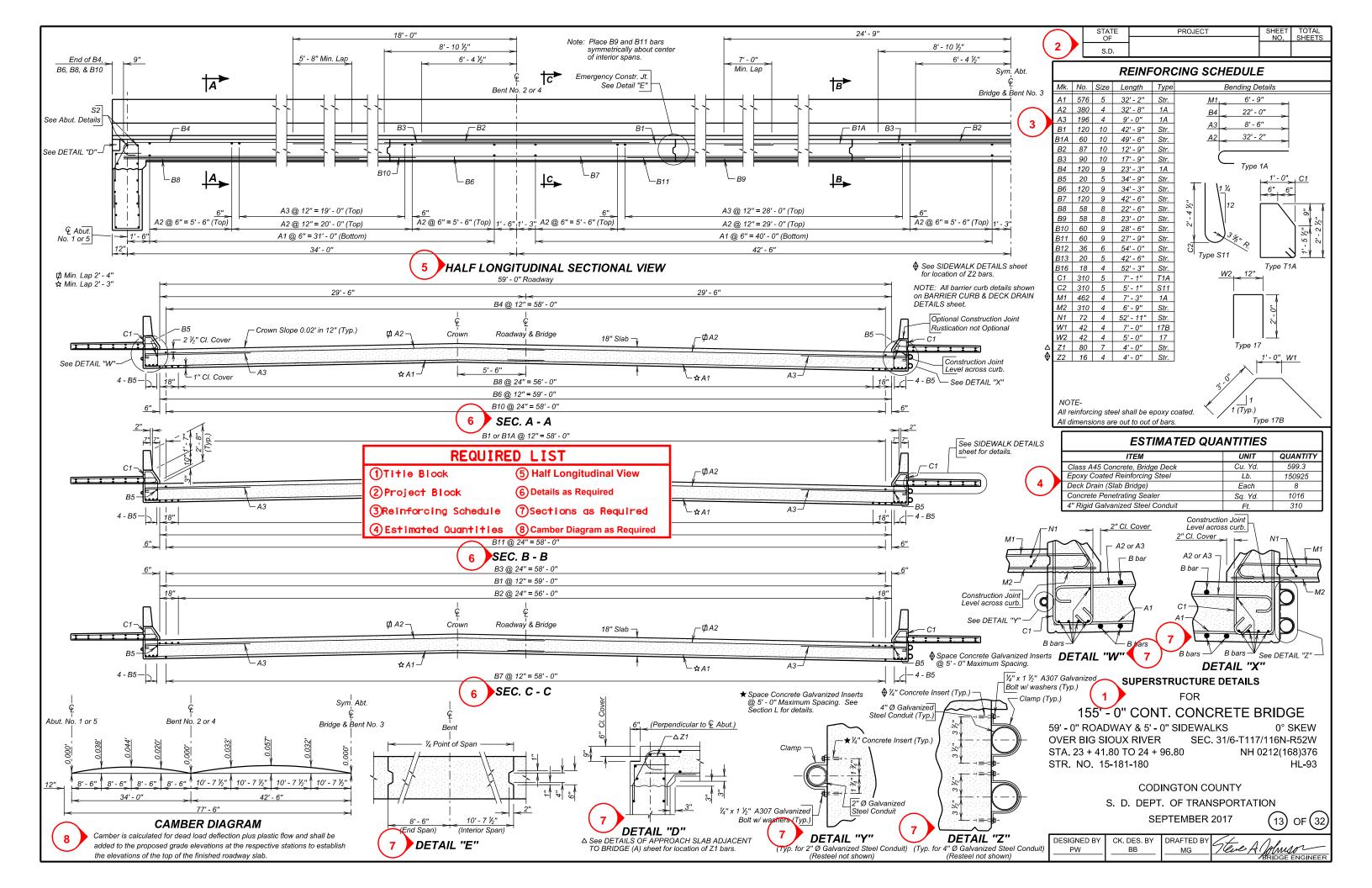
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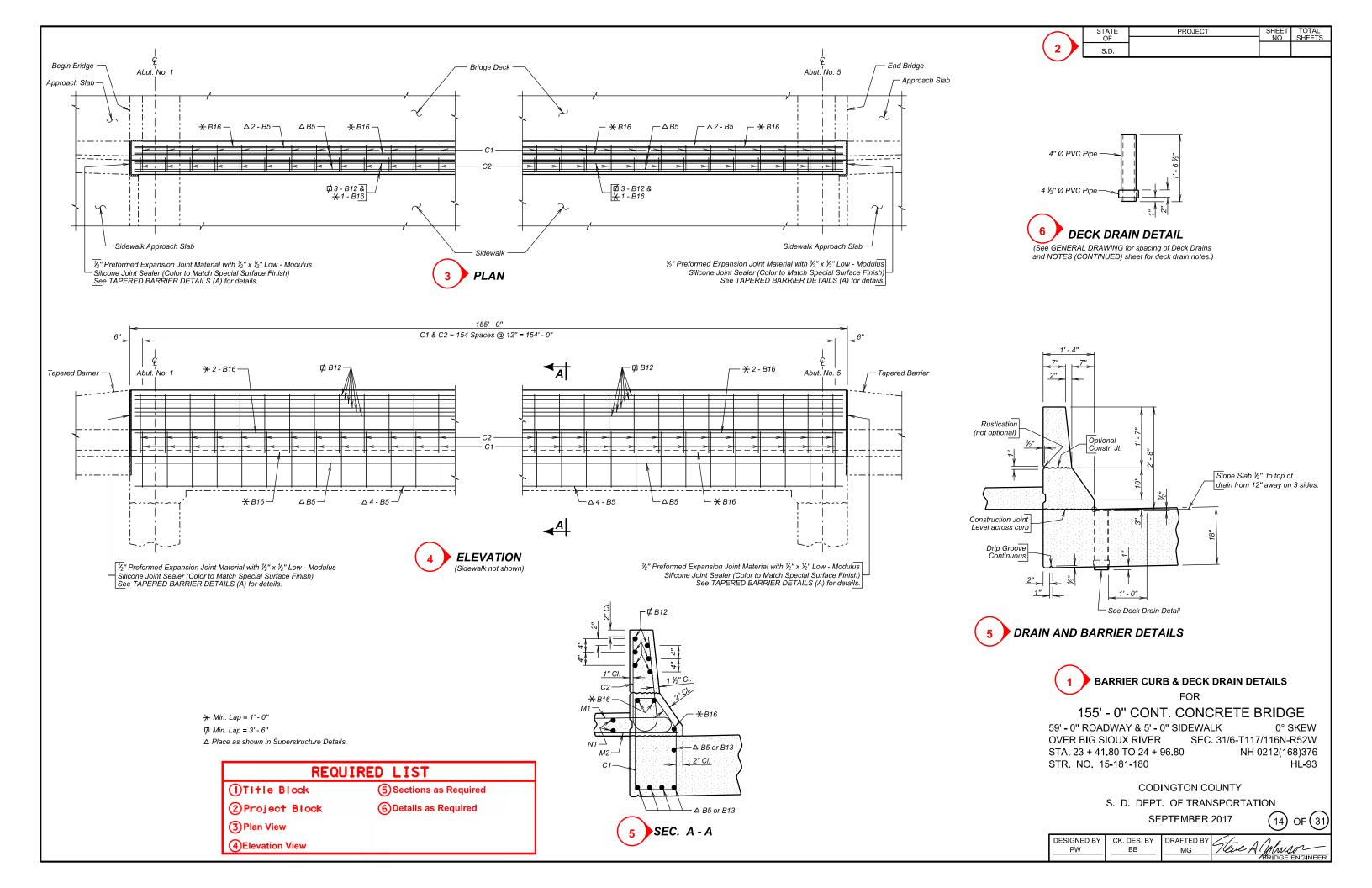
SEPTEMBER 2017

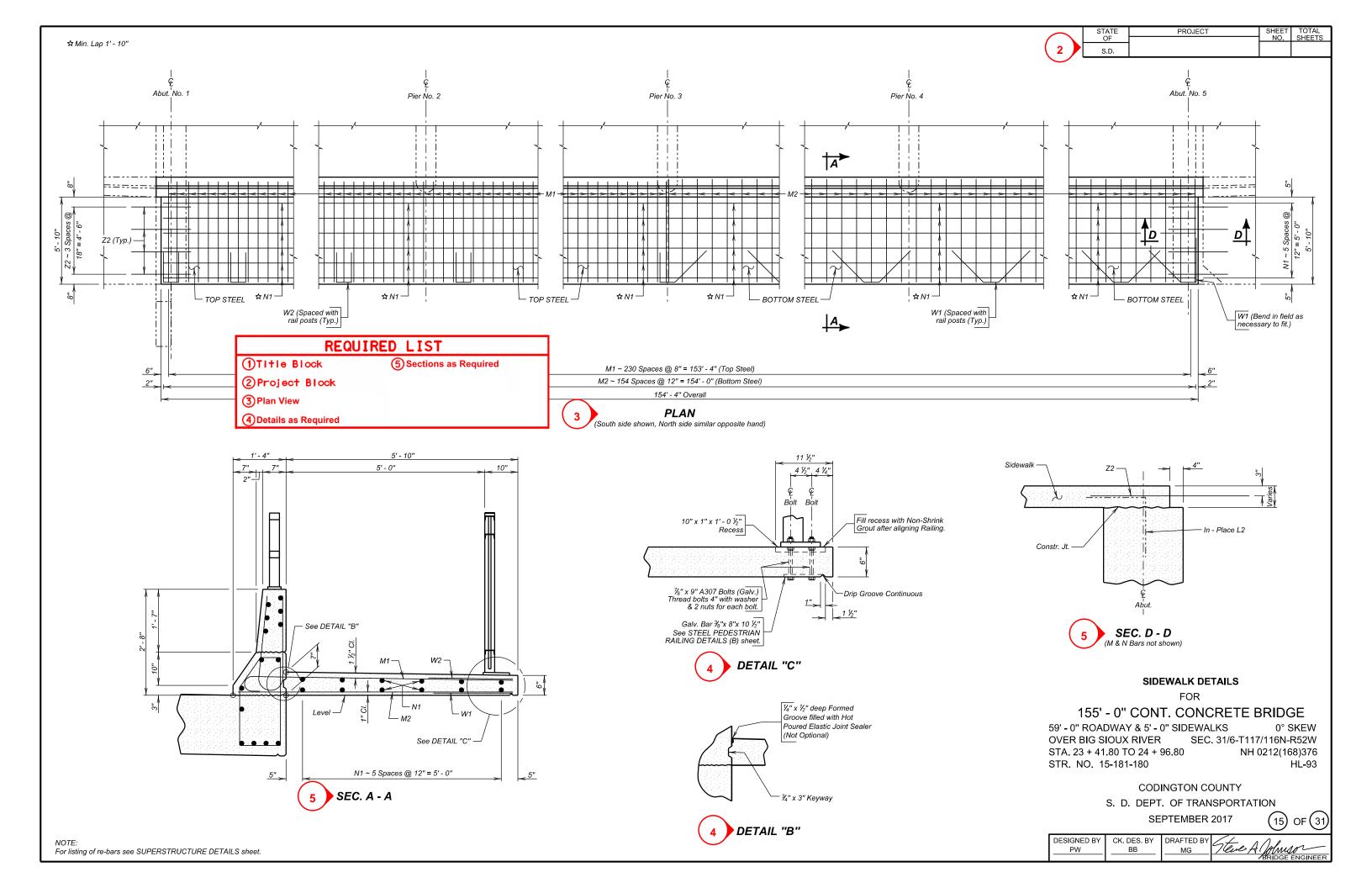
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PW	BB	MG	Mene A Johnson
			Bridge engineer

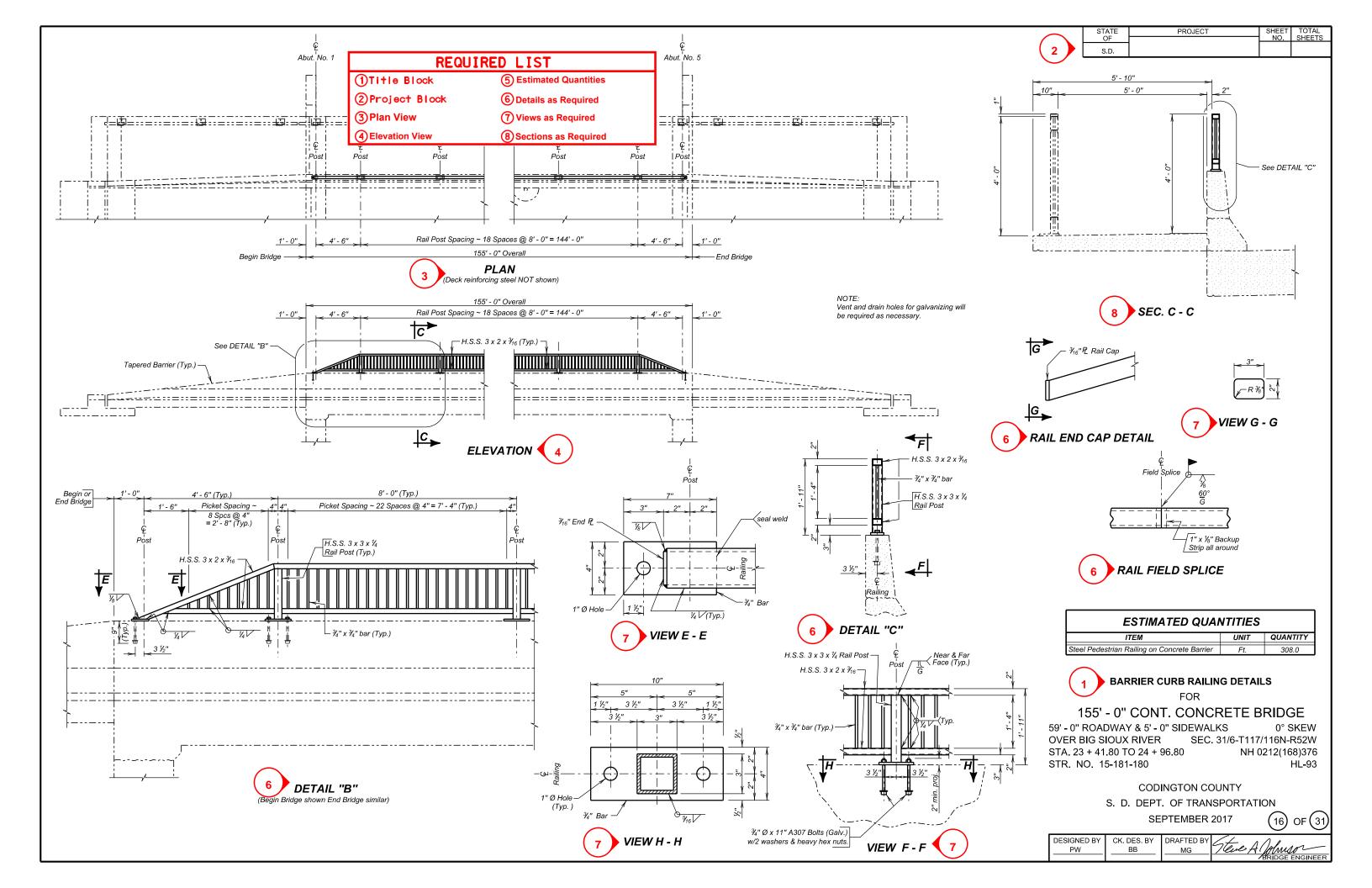


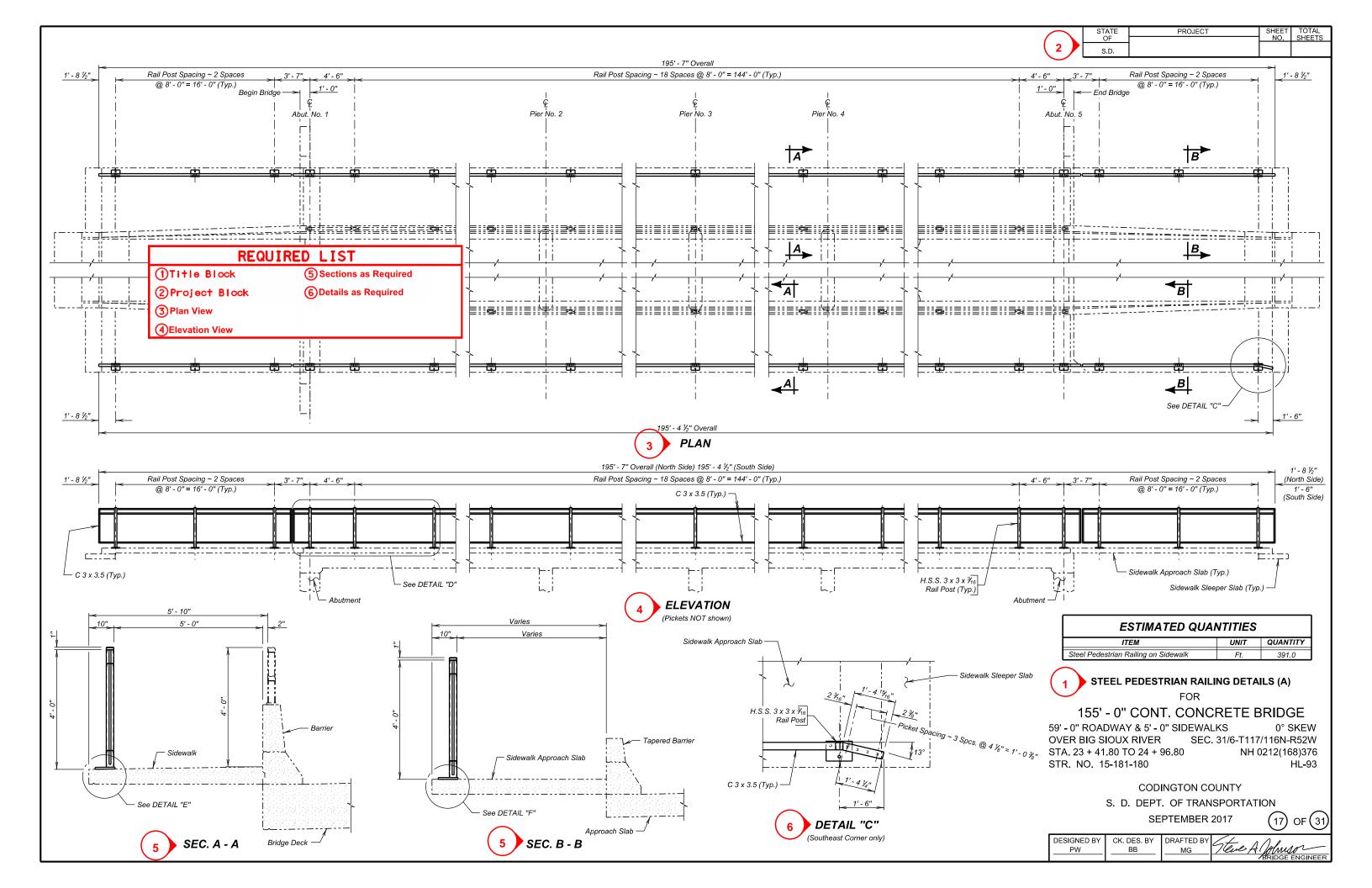


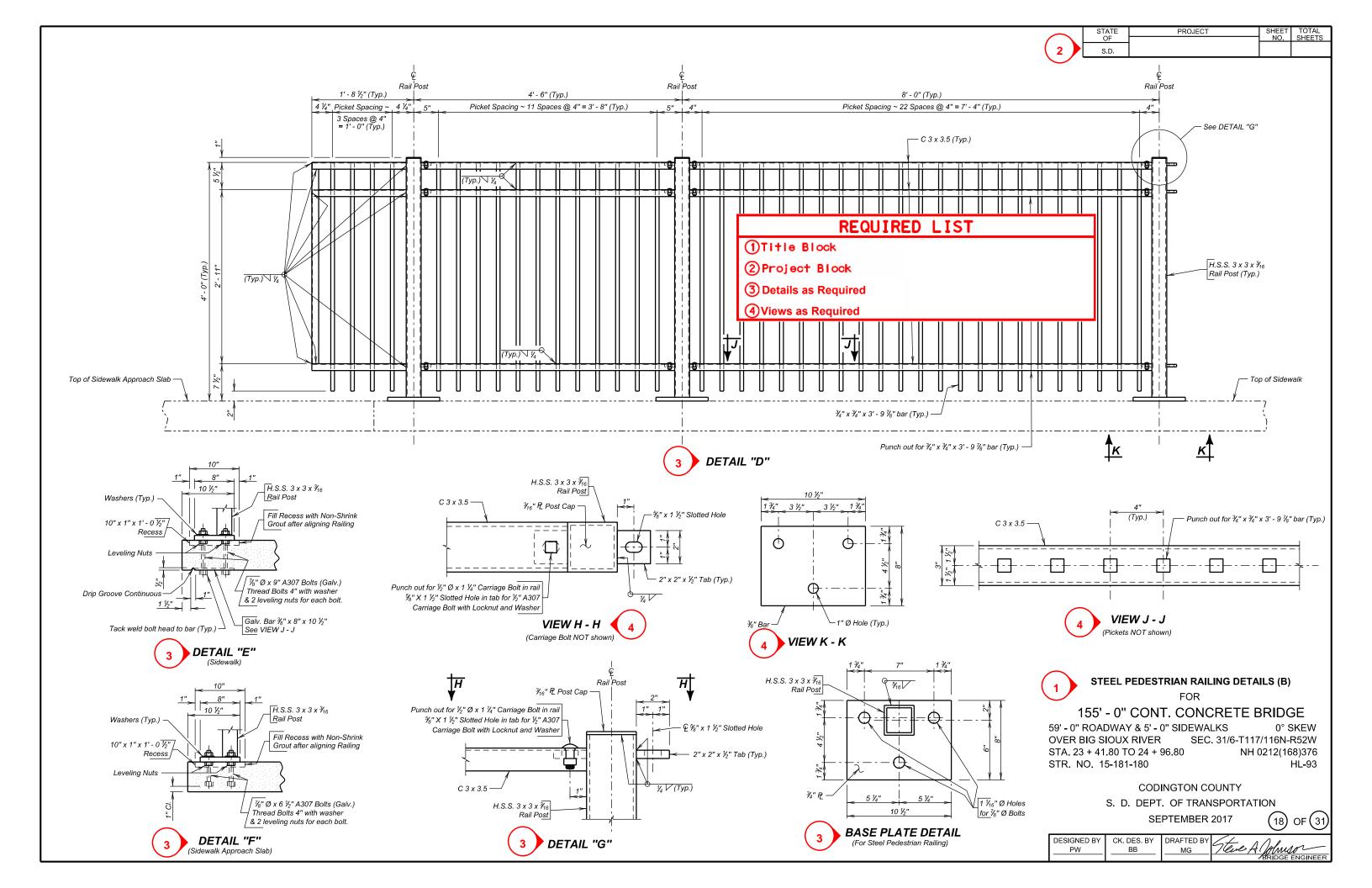


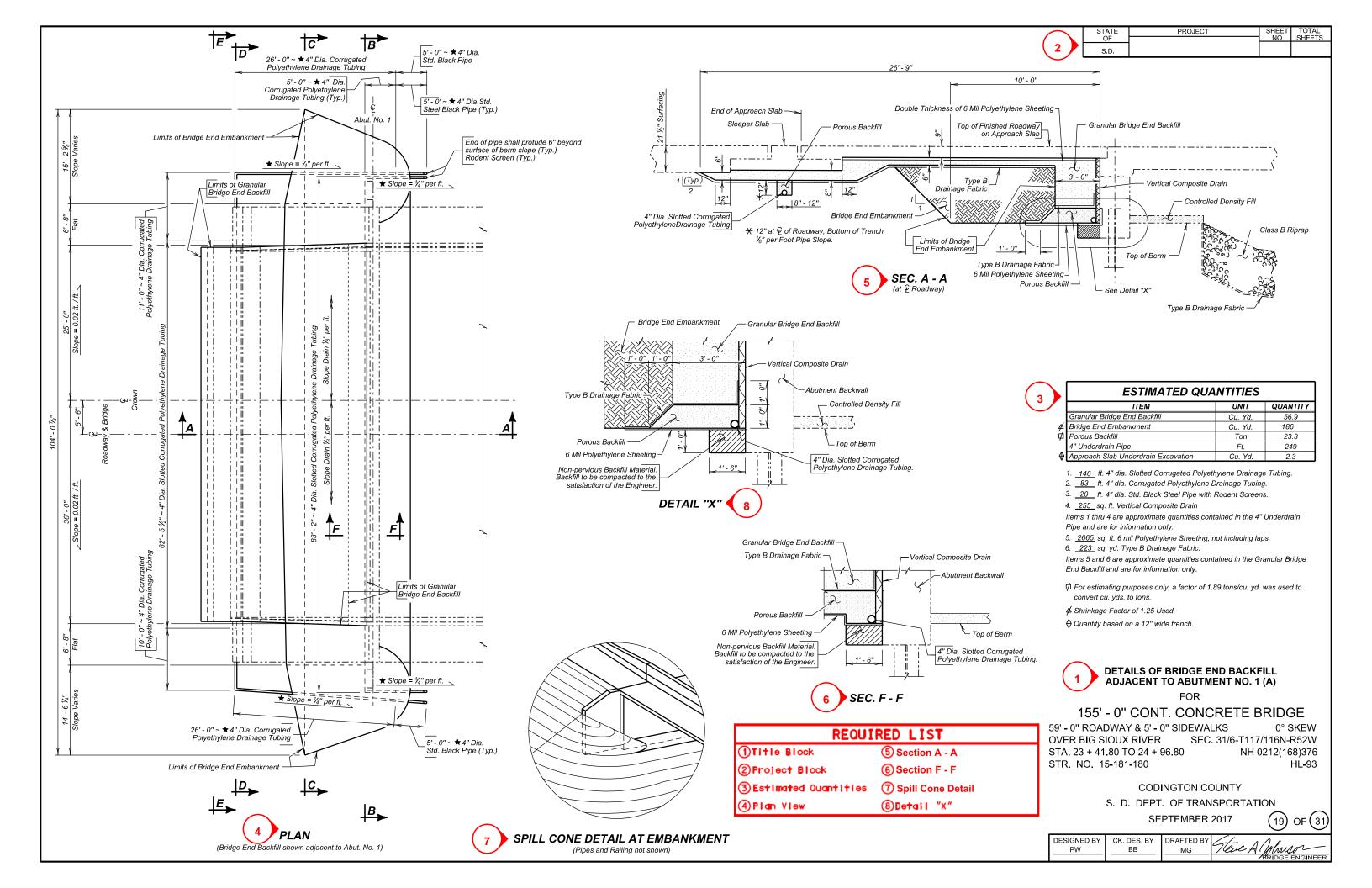


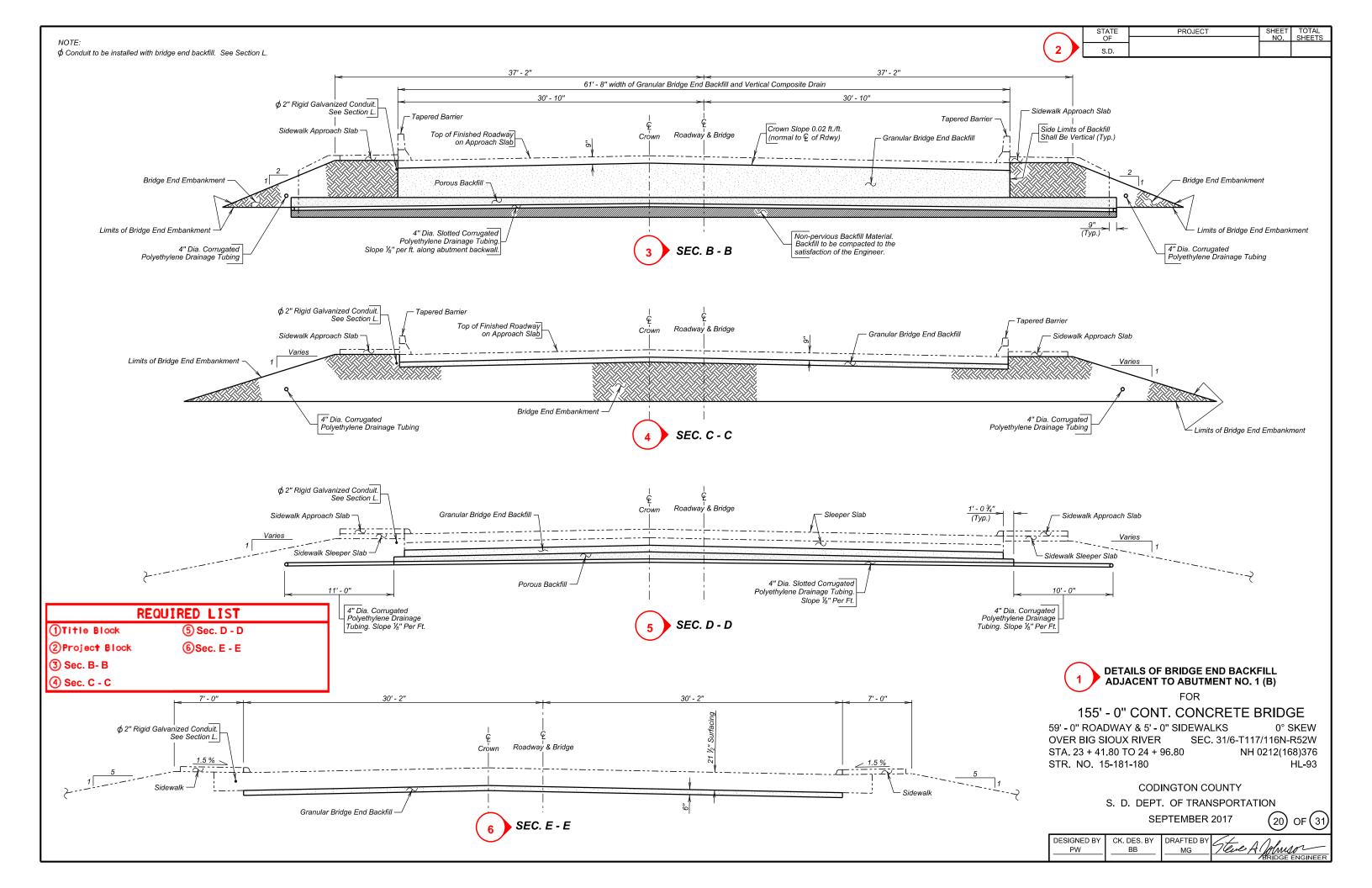


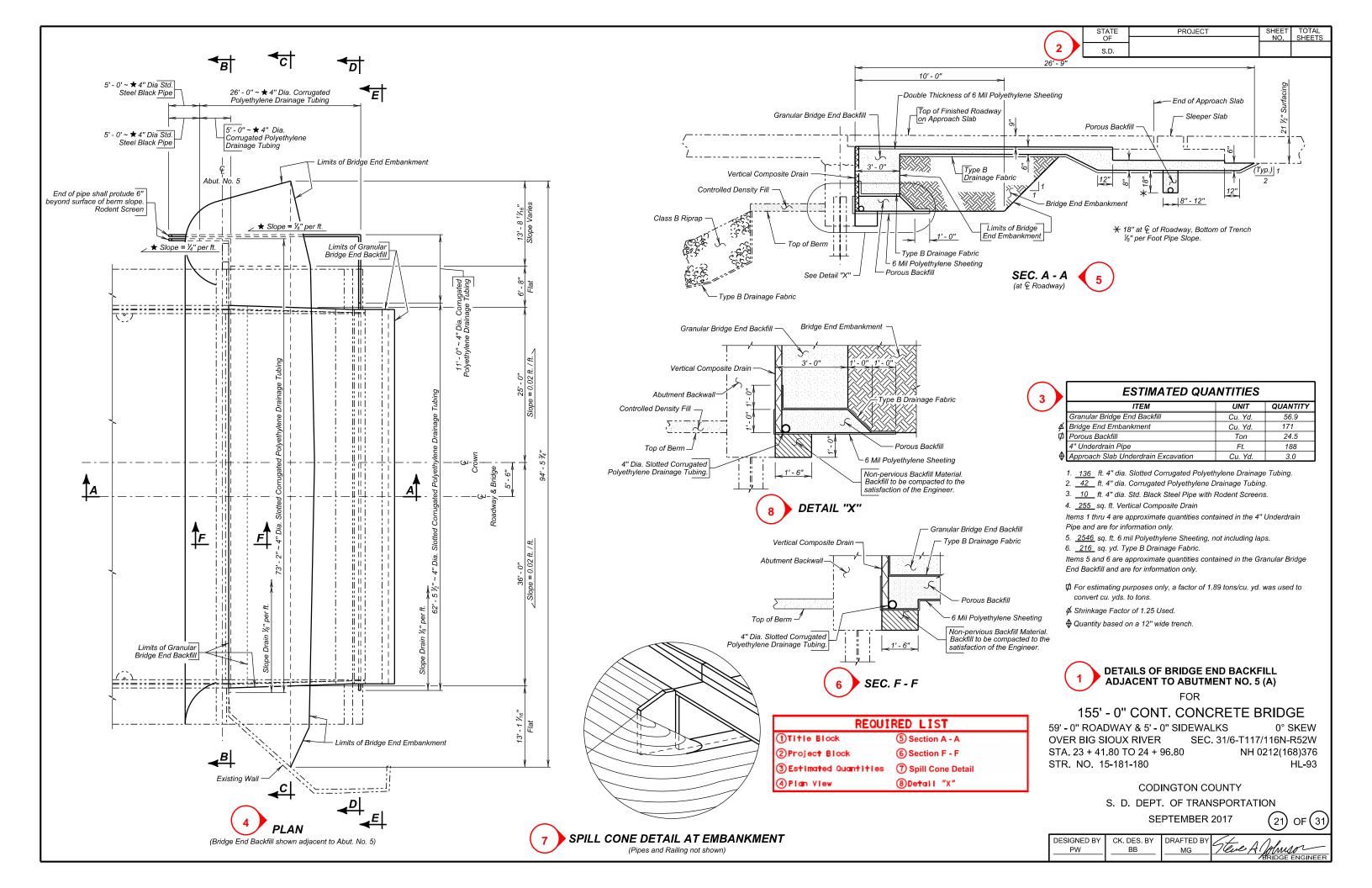


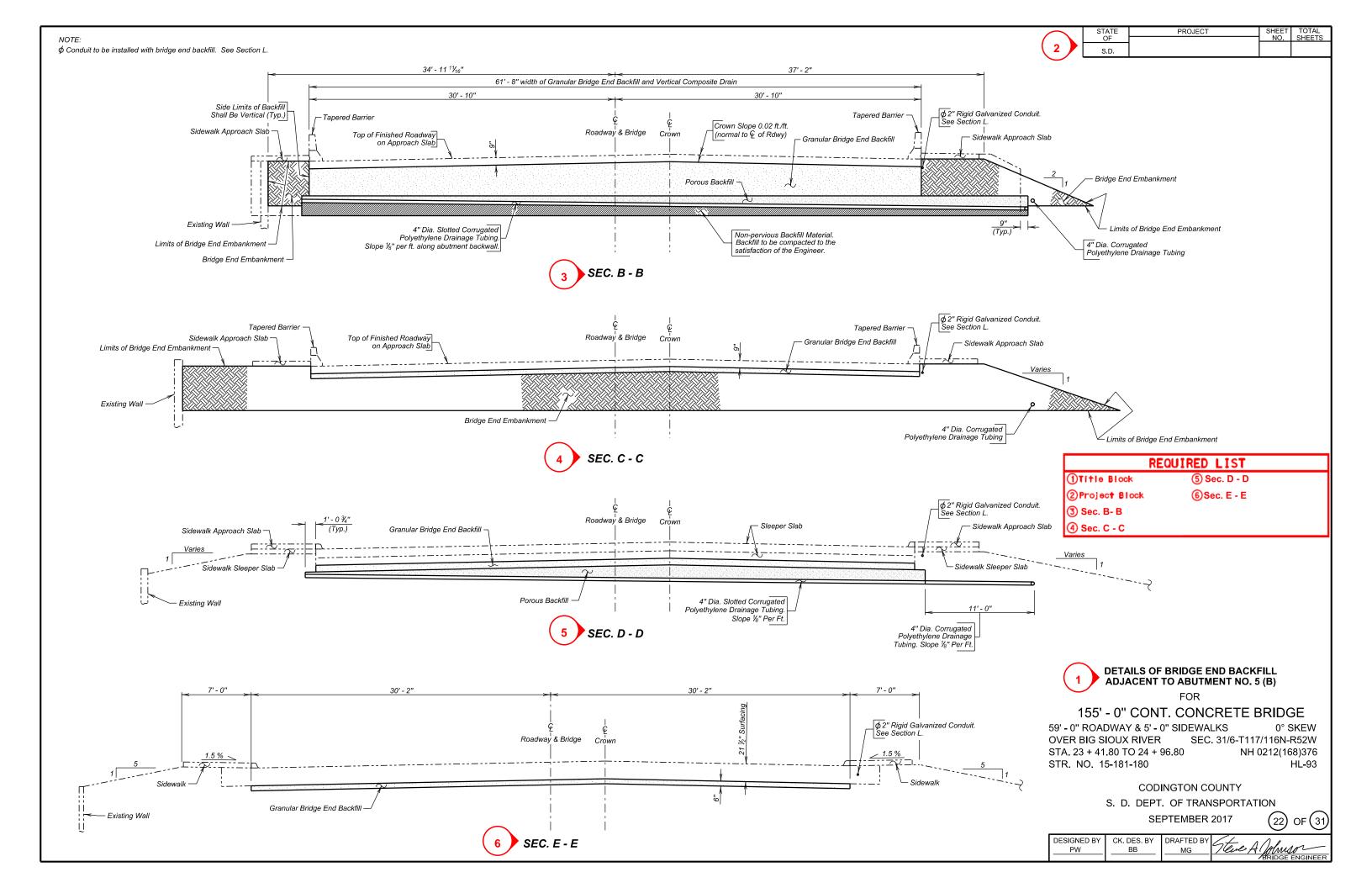


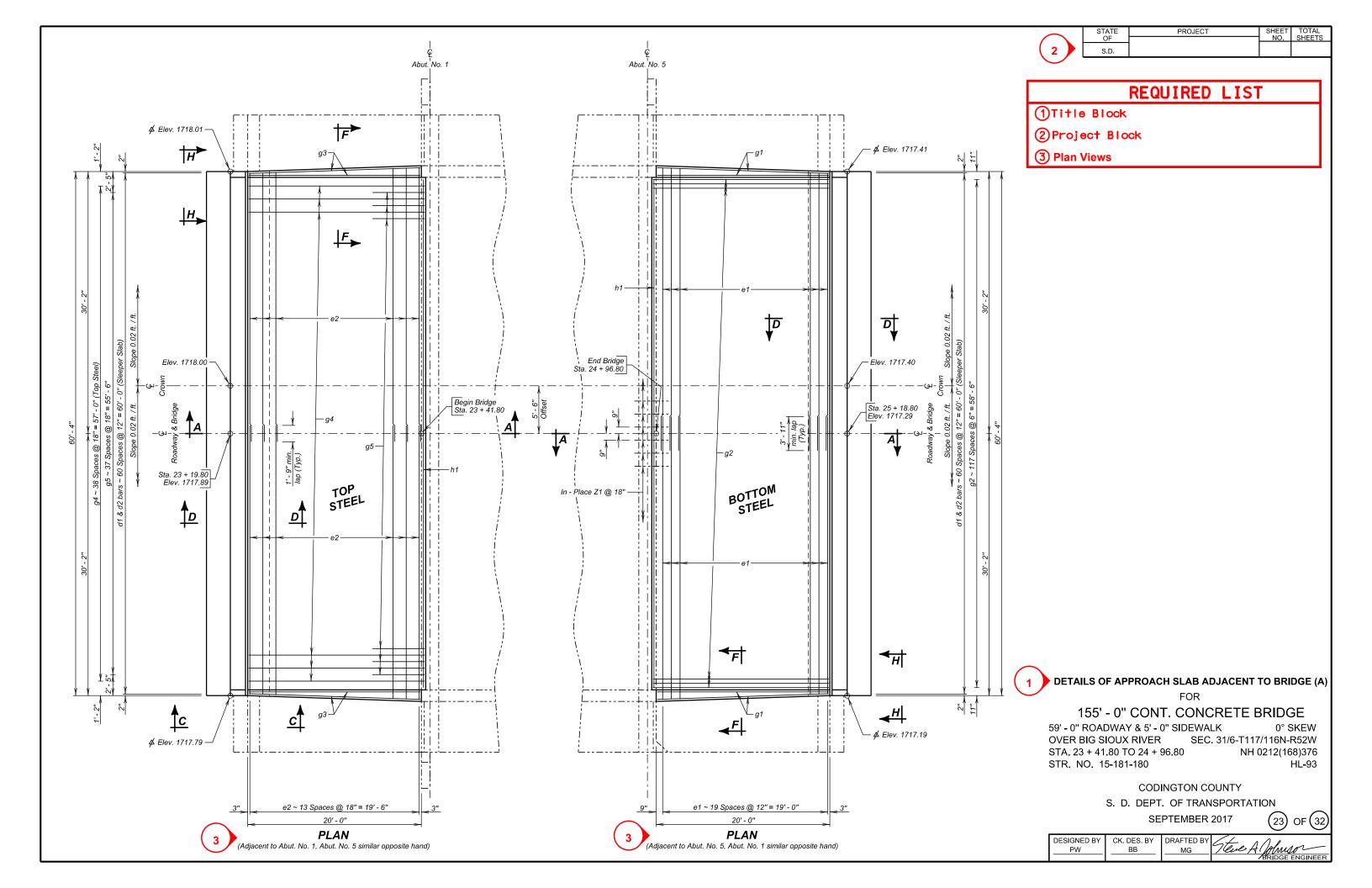




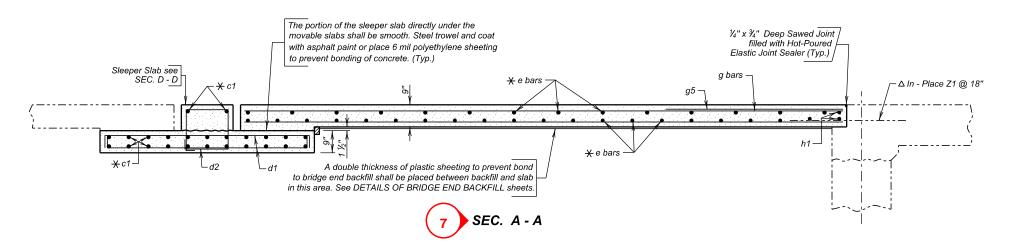


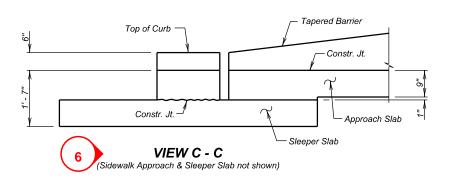


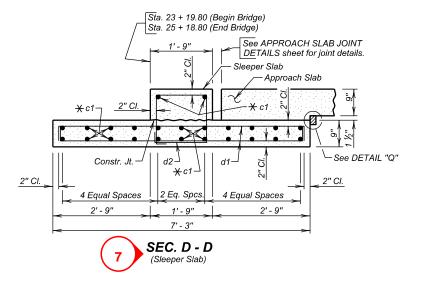




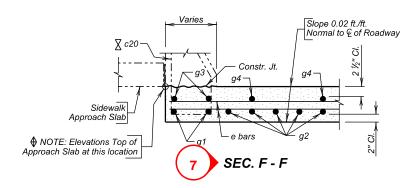


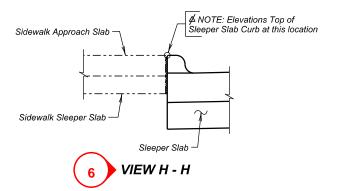


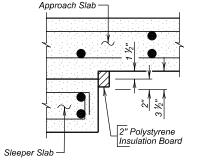




X Shown and included in reinforcing schedule for tapered barriers. See TAPERED BARRIER DETAILS (B) sheet for details.







DETAIL "Q"

PROJECT S.D.

REINFORCING SCHEDULE (For Two Approach Slabs & Two Sleeper Slabs) Mk. No. Size Length Type Sleeper Slabs c1 96 5 31'-4" Str. 
 d1
 244
 4
 7'-9"
 2

 d2
 122
 4
 6'-2"
 72

Approach Slabs e1 80 6 32' - 4" Str. e2 56 4 31'-7" Str. g1 8 8 19'-9" Str. g2 | 236 | 8 | 20' - 3" | Str. g3 | 8 | 4 | 19' - 9" | Str. g4 78 4 20'-3" Str. g5 76 4 6'-0" Str. h1 4 6 58'-6" Str.

d2 1' - 5" Type T2 d1 \_ 6' - 11" \_ Type 2

NOTE:

All bars to be epoxy coated. All dimensions are out to out of bars.

**ESTIMATED QUANTITIES** 

(For Two Approach Stabs and Two Steeper Stabs)			
UNIT	QUANTITY		
Sq. Yd.	277.7		
Sq. Yd.	97.3		
	UNIT Sq. Yd.		

70.8 Cu. Yds. Concrete in Approach Slabs.

20065 Lbs. Epoxy Coated Re-Steel in Approach Slabs.

31.1 Cu. Yds. Concrete in Sleeper Slabs.

4903 Lbs. Epoxy Coated Re-Steel in Sleeper Slabs.

30 Sq. Ft. of 2" Polystyrene Insulation Board.

6. 3.8 Cu. Yds. Concrete in Tapered Barriers. 7. 1072 Lbs. Epoxy Coated Re-Steel in Tapered Barriers.

Items 1 thru 7 are approximate quantities contained in the above bid items and are for information only.

#### REQUIRED LIST

(1)Title Block

(5) Details as Required

(2) Project Block

**(6)** Views as Required

(3)Reinforcing Schedule

(7)Sections as Requried

(4) Estimated Quantities

#### DETAILS OF APPROACH SLAB ADJACENT TO BRIDGE (B) FOR

#### 155' - 0" CONT. CONCRETE BRIDGE

59' - 0" ROADWAY & 5' - 0" SIDEWALK OVER BIG SIOUX RIVER SEC. 31/6-T117/116N-R52W STA. 23 + 41.80 TO 24 + 96.80 NH 0212(168)376 STR. NO. 15-181-180

CODINGTON COUNTY

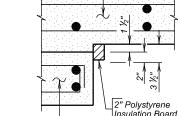
S. D. DEPT. OF TRANSPORTATION

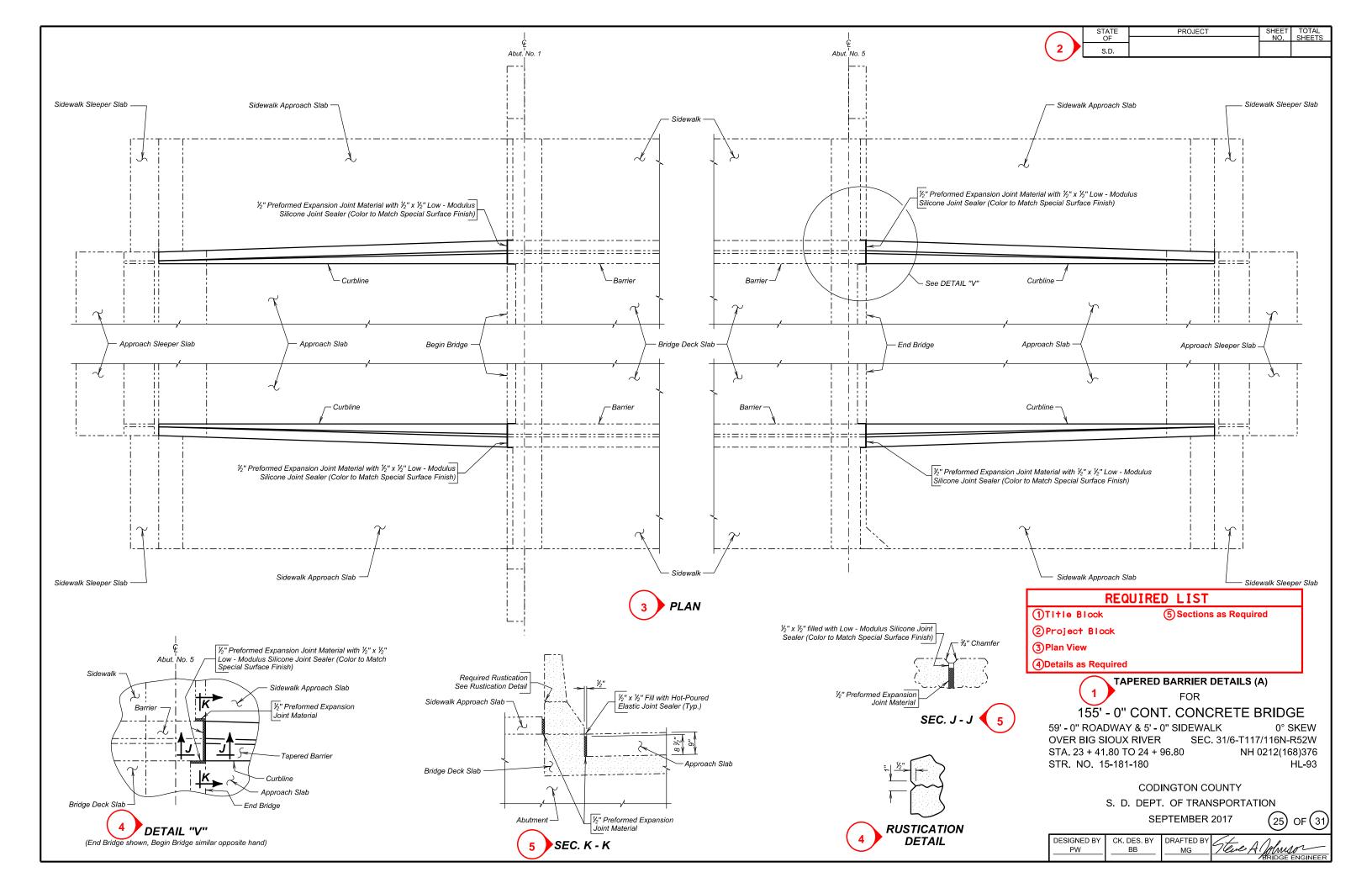
SEPTEMBER 2017

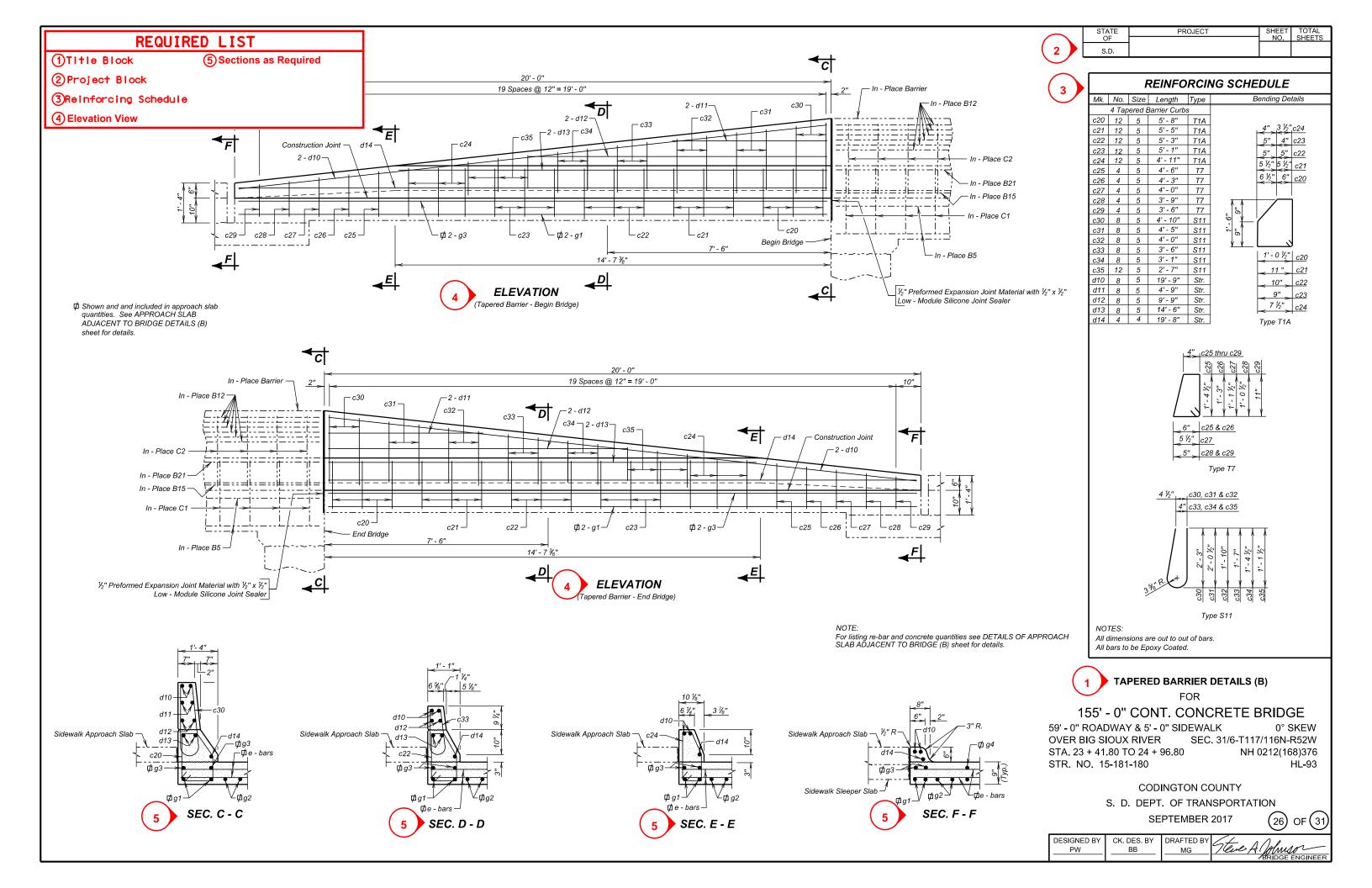
(24) OF (31

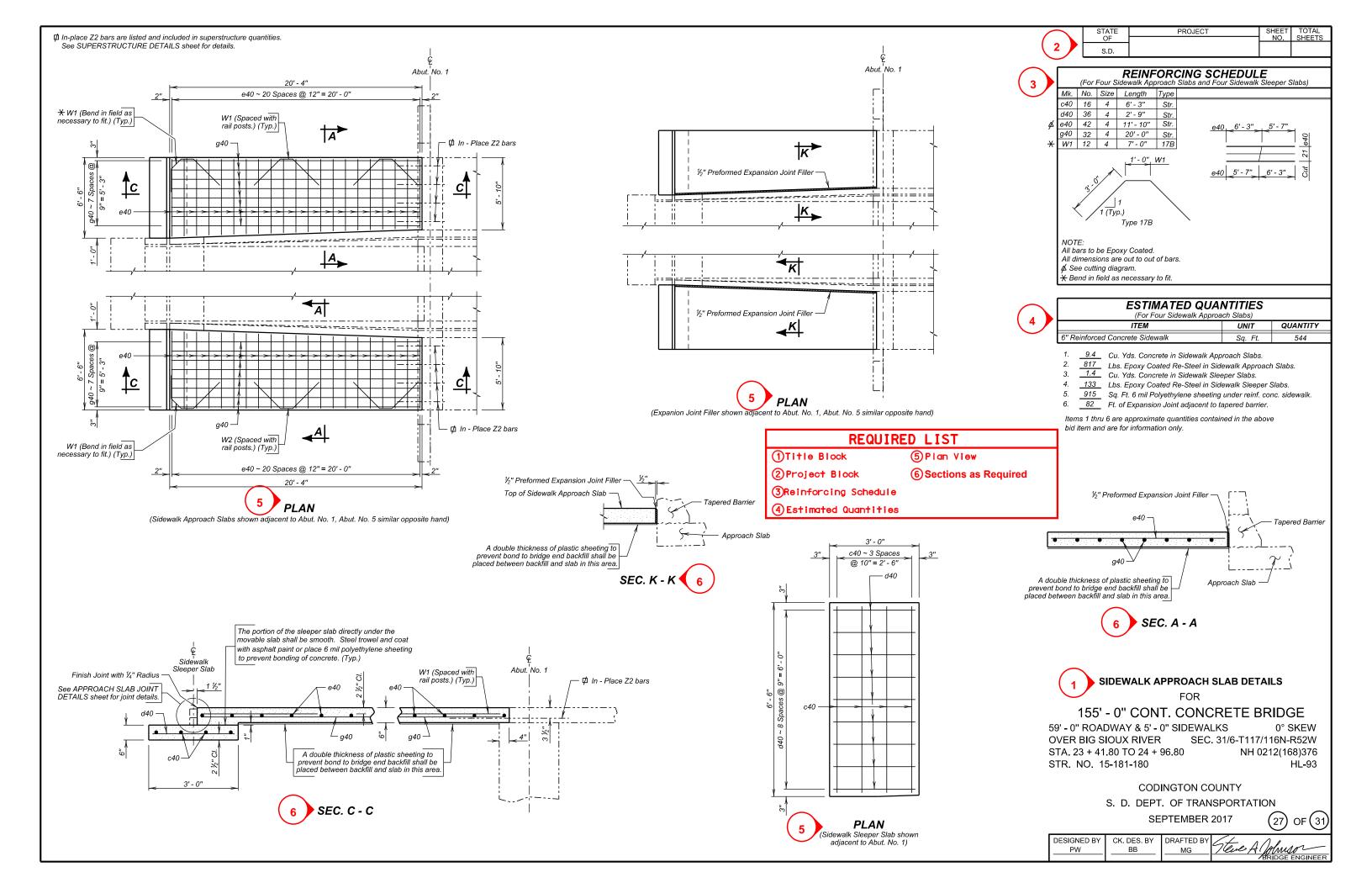
DESIGNED BY CK. DES. BY DRAFTED BY MG

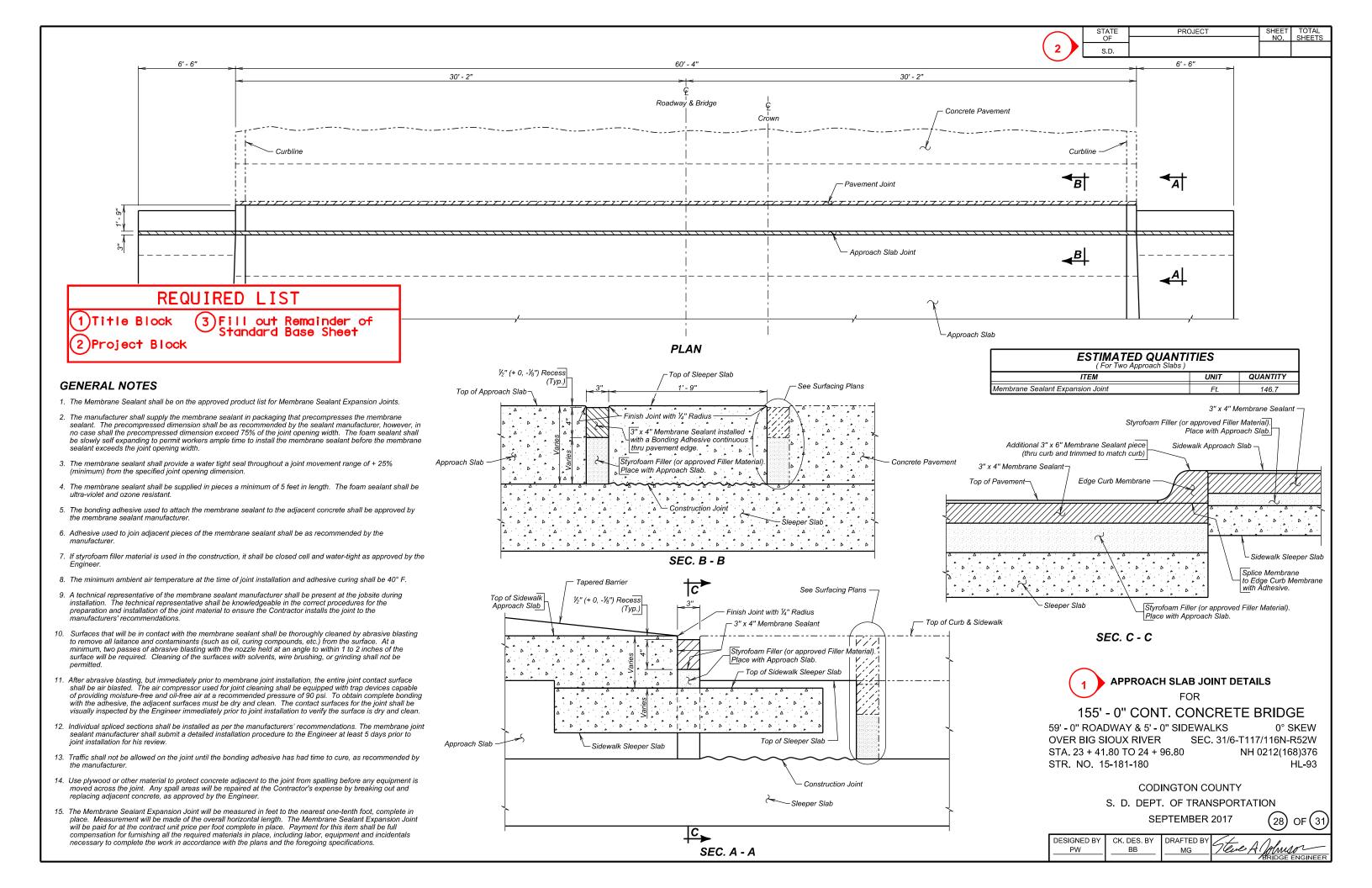


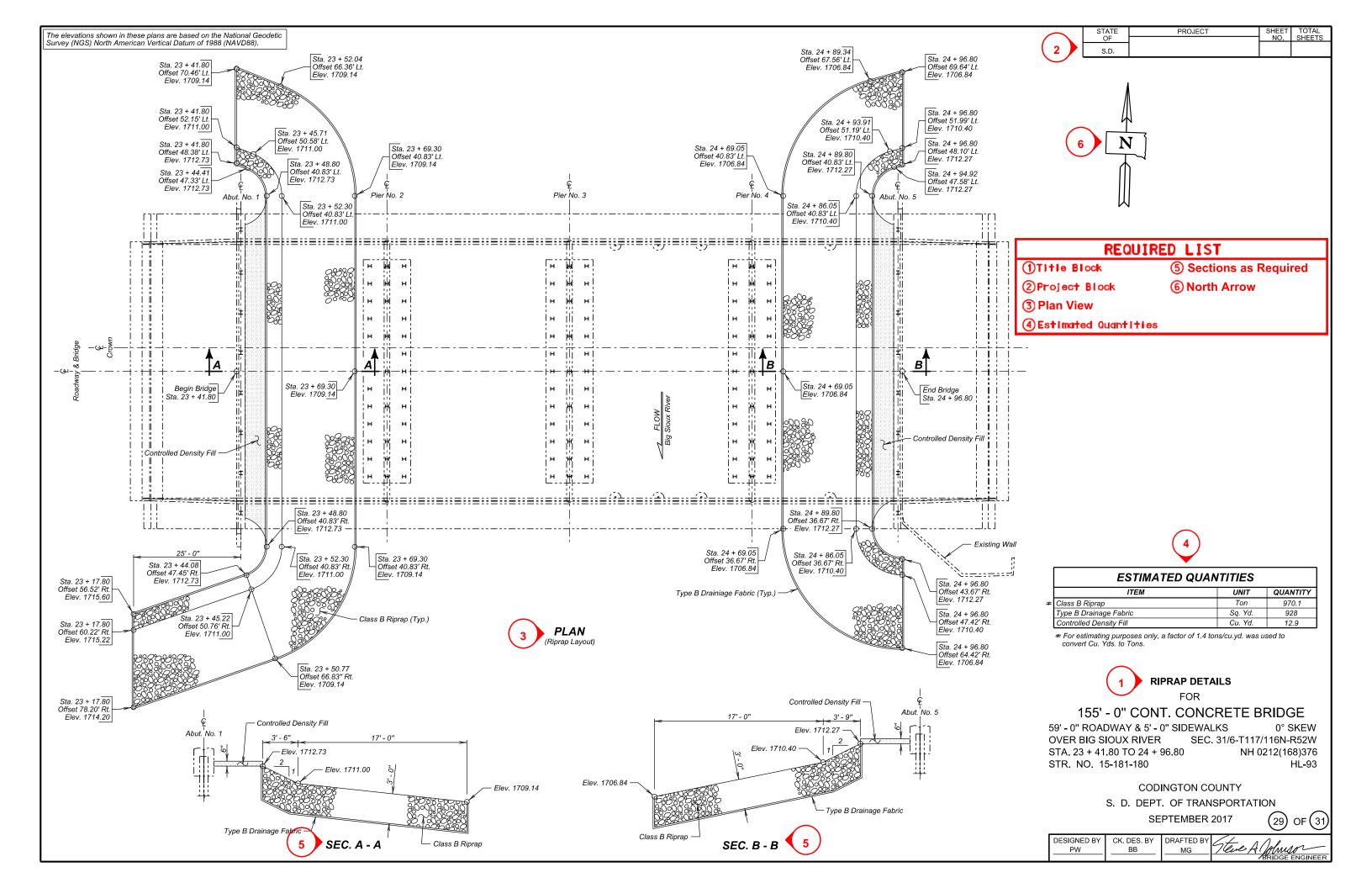




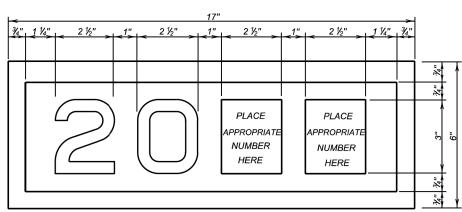








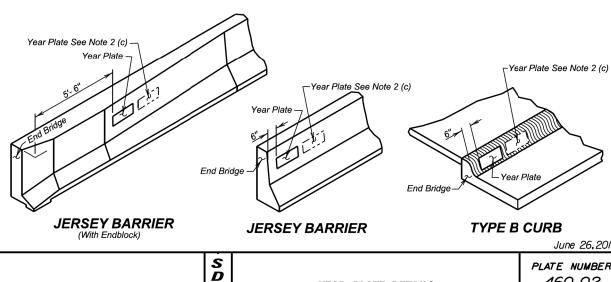




#### YEAR PLATE DETAILS

#### **GENERAL NOTES:**

- 1. Year plates of the general dimensions shown shall be constructed on all box culverts and bridges. The year plates shall be constructed in reverse and attached to the forms in such a manner that the finished imprint in the concrete does not exceed one-half (1/2) inch in depth.
- 2. Year plates shall be located on structure (s) as follows:
- a. On cast-in-place box culverts the year plates shall be four and one half (4 1/2) inches below the top of the upstream parapet wall and centered laterally on the upstream face. On precast box culverts the year plate shall be centered laterally on the upstream face of the top slab. Where an extended interior wall interferes with this location, the year plate shall be centered in an adjacent barrel.
- b. On bridges with six (6) inch curbs or "Jersey" shaped barriers with no endblocks, the year plate shall be centered vertically on the curb face approximately six (6) inches from the end of the bridge, or as designated by the Engineer. On bridges with "Jersey" shaped barrier endblocks, the year plate shall be centered on the upper sloped portion of the barrier approximately 5'- 6" from the end of the bridge, or as designated by the Engineer. There shall be one year plate at each end of the bridge on opposite sides.
- c. When the plans specify that both the original date of construction and the date of reconstruction are to be shown, one date shall be placed as listed above and the other located adjacent to it. Both year plates shall be shown at each end of the bridge on opposite sides.
- 3. There will be no separate measurement or payment made for year plates on box culverts and bridges. All costs for this work shall be incidental to other contract items.



Published Date: 1st Qtr. 2019

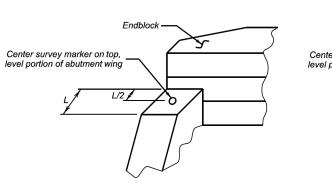
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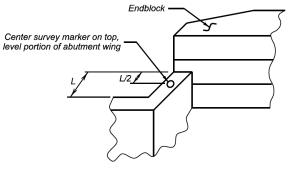
YEAR PLATE DETAILS

PLATE NUMBER 460.02

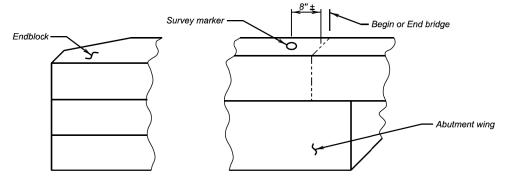
Sheet I Of I



**ABUTMENT WITH** "STRAIGHT" WINGS



ABUTMENT WITH "SWEPT BACK" WINGS



#### **ABUTMENT WITH** "SWEPT BACK" WINGS

( Endblock on top of wings)

#### **GENERAL NOTES:**

- 1. Survey markers shall be located at each abutment on the same side of the bridge as the year plate. Place survey markers on abutment wings as shown. Two survey markers will be required at each bridge
- 2. Survey markers shall be of a type intended for installation in concrete, be made of solid brass or bronze, have a domed top and be either a 3" top diameter (with a ¾" X 2" long ribbed shank), or a US Army Corps of Engineers Type C Disc with a 3 1/2" top diameter.
- 3. There will be no separate measurement or payment made for survey markers. All costs for this work shall be incidental to the other contract items.

June 26,2012

D  $\bar{D}$ 0 Published Date: 1st Qtr. 2019

BRIDGE SURVEY MARKER

PLATE NUMBER 460.05

Sheet I of I

REQUIRED LIST

3 Insert Required Standard Plate Sheets as Needed 1)Title Block (2)Project Block



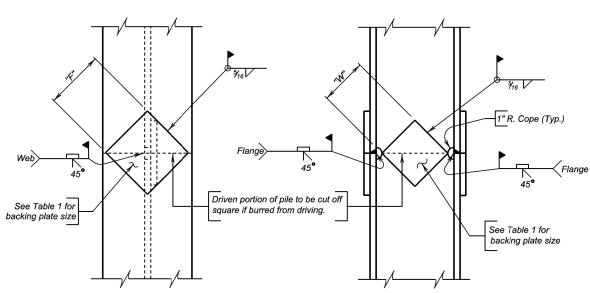
155' - 0" CONT. CONCRETE BRIDGE



**2** 

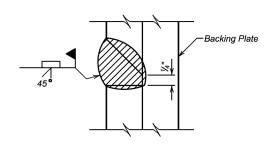
STATE PROJECT
OF
S.D.

SHEET TOTAL NO. SHEETS



#### **COMPLETE JOINT PENETRATION WELD DETAIL**

Prepare joint surfaces lower end of upper section on the ground and weld on backing plates; then place upper section on lower section and weld.



#### **GENERAL NOTES:**

- 1. Steel for backing plates shall conform to ASTM A709 Grade 50.
- Welding and weld inspection shall be in conformance with AWS D1.5 (Current Year) Bridge Welding Code Steel.
- 3. Welder must be certified and registered with the SDDOT.
- 4. Backing plate shall at a minimum be as thick as the web of the pile being spliced.
- 5. Web must be coped with 1 inch radius.

Published Date: 1st Qtr. 2019

Submit Welding Procedure Specification (WPS) to Bridge Construction Engineer for approval prior to pile driving.

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TABLE 1 (BACKING PLATES)			
PILE	10"	12"	14"
"F" FLANGE	6 ½"	8"	10"
"W" WEB	4 ¾"	6 ¼"	7 ½"

STEEL PILE SPLICE DETAILS

PLATE NUMBER 5/0.40

December 23,2012

Eyebolt (Typ.) (See EYEBOLT DETAILS)

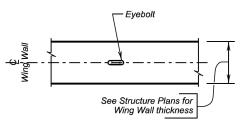
ABUTMENT WING

**DETAIL FOR FENCE ANCHORS** 

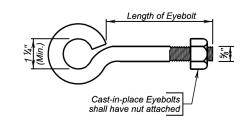
#### **GENERAL NOTES:**

Published Date: 1st Qtr. 2019

- The fence and post details shown are for illustrative purpose only. The fence shall be as specified elsewhere in the plans.
- 2. Eyebolts shall be placed on all of the bridge abutment wings.
- 3. Eyebolts shall be  $\frac{4}{8}$  inch diameter and shall conform to ASTM A307.
- Eyebolts, nuts, and concrete inserts shall be galvanized in accordance with AASHTO M232 (ASTM A153). Concrete inserts of corrosion resistant material need not be galvanized.
- 5. Cast-in-place eyebolts shall have a nut attached, be 4 ½ inches (Min.) in length and shall be embedded such that the eye of the bolt is flush with the concrete surface. (See Eyebolt Details) As an alternate, cast-in-place concrete inserts, capable of developing the full strength of the ¾ inch diameter threaded eyebolt, may be used and shall be set in the concrete in accordance with the manufacturer's recommendations. The eyebolt shall be of sufficient length to develop its full strength. The eye of the eyebolt shall be flush with the concrete surface.
- The cost for furnishing and installing eyebolts and/or concrete inserts shall be incidental to various contract items.



VIEW A - A



**EYEBOLT DETAILS** 

December 23,2012

FENCE ANCHORS FOR BRIDGE ABUTMENT WINGS (WINGS 6' AND SHORTER)

PLATE NUMBER 620.18

Sheet I of I

REQUIRED LIST

1) Title Block 3 Insert Required Standard Plate Sheets as Needed 2) Project Block



155' - 0" CONT. CONCRETE BRIDGE